## SULIT

#  <br> Second Semester Examination <br> 2021/2022 Academic Session <br> July/August 2022 <br> EAH325 - Engineering Hydrology 

Duration : 2 hours

Please ensure that this examination paper consists of NINE (9) pages of printed material before you begin the examination.

Instructions: This paper contains FIVE (5) questions, answer any FOUR (4) questions.

All questions MUST BE answered on a new page.

1. (a) Describe THREE (3) factors that will affect the evaporation process.
[6 marks]
(b) A Class A pan was set up adjacent to a lake. At the beginning of a certain week, the water level inside the pan was 203 mm . Later in that week, there was a 41 mm of rainfall water. Subsequently, 22 mm of water was removed from the pan to keep the water level within the specified depth range. Calculate the pan evaporation and the lake evaporation if the depth of the water in the pan at the end of the week is 189 mm . Use a suitable pan coefficient.
[5 marks]
(c) A reservoir with an average surface spread of $4.5 \mathrm{~km}^{2}$ in March has a water surface temperature of $20^{\circ} \mathrm{C}$ and relative humidity of $27 \%$. Wind velocity measured at 2.0 m above the ground at a nearby observatory is $35 \mathrm{~km} / \mathrm{h}$. Calculate the average evaporation loss from the reservoir in $\mathrm{mm} /$ day, the total depth, and volume of evaporation loss for March.
2. (a) With the aid of sketches, describe the following terms:
i) Saturated zone
ii) Unsaturated zone
iii) Aquifuge
iv) Hydraulic conductivity
v) Pumping test
(b) The saturated medium has a porosity of 0.45 and discharge per unit area of $1 \times 10^{-5} \mathrm{~m}^{3} / \mathrm{s} / \mathrm{m}^{2}$. The water level in two observation wells at a distance of 400 m apart are 12 m and 8 m for higher end and lower end, respectively. Determine the hydraulic conductivity of the medium.
[10 marks]
3. (a) There are factors to be considered in the selection of method for water level recording station. With the aid of a sketch, select and explain the most appropriate method that is recommended for discharge measurement near hilly area.
[10 marks]
(b) Table 1 shows the characteristics of a river reach of 1.5 km apart during a high flood event. Given velocity head coefficient, 1.12 and loss coefficient, 1.0. Calculate the discharge of the river.

## TABLE 1

| Upstream: | Cross-sectional area | $\mathrm{A}_{1}$ | $=$ | $170 \mathrm{~m}^{2}$ |
| :--- | :--- | :---: | :--- | :--- |
|  | Wetted perimeter | $\mathrm{P}_{1}$ | $=$ | 50 m |
|  | Manning's roughness coefficient | $\mathrm{n}_{1}$ | $=$ | 0.03 |
|  | Reduced level of water |  | $=77.3 \mathrm{~m}$ |  |


| Downstream: | Cross-sectional area | $\mathrm{A}_{2}$ | $=$ | $175 \mathrm{~m}^{2}$ |
| :--- | :--- | :---: | :---: | :---: |
|  | Wetted perimeter | $\mathrm{P}_{2}$ | $=$ | 51 m |
|  | Manning's roughness coefficient | $\mathrm{N}_{2}$ | $=$ | 0.025 |
|  | Reduced level of water |  | $=$ | 77.0 m |

4. The 2-hr Unit Hydrograph for a river basin in Northern Seberang Perai is given in Table 2. Determine the catchment area and volume of direct runoff due to effective rainfall given in Table 3. Use superposition method in the determination of direct runoff hydrograph.
[25 marks]

Table 2 The 2-hr Unit Hydrograph

| Ordinate <br> (hr) | 0 | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 | 11 | 12 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| UH <br> $\left(\mathrm{m}^{3} / \mathrm{s} / \mathrm{cm}\right)$ | 0 | 5 | 18 | 28 | 35 | 45 | 47 | 37 | 25 | 16 | 9 | 3 | 0 |

Table 3 Effective Rainfall Hyetograph

| Time (hr) | Rainfall Depth (mm) |
| :---: | :---: |
| $0-2$ | 50 |
| $2-4$ | 90 |
| $4-6$ | 60 |

5. (a) The platform for a residential development is designed for 100 years (return period) flood. Determine the following for the residential area:
i) The exceedance probability of 100 years flood.
ii) The probability of no flooding in 10 years period
iii) The probability of no flooding in the first 5 years and flooding in the $6^{\text {th }}$ year.
iv) Probability at least one flood will occur in 10 years.
v) Determine the return period if the risk is $20 \%$ that the residential area will be flooded at least once during the next 5 consecutive years.
[10 marks]
(b) Local authority plans to construct a road in Southern Seberang Perai. The design of the platform of the road requires some analysis on the flood probability and magnitude. The mean and variance of the 100 years of annual streamflow record in the locality are $120 \mathrm{~m}^{3} / \mathrm{s}$ and 30 $\mathrm{m}^{6} / \mathrm{s}^{2}$, respectively. Assuming the data is log-normally distributed, determine the followings:
i) the return period for a streamflow discharge $\geq 150 \mathrm{~m}^{3} / \mathrm{s}$.
ii) the magnitude of 50-year return period flood.

## ATTACHMENT

| Types of pan | Average value | Range |
| :--- | :--- | :--- |
| Class A Pan | 0.70 | $0.60-0.80$ |
| Colorado Sunken Pan | 0.78 | $0.75-0.86$ |
| USGS Floating Pan | 0.80 | $0.70-0.82$ |

## Formulas

$$
\begin{gathered}
e_{w}=4.584 \exp \left(\frac{17.27 t}{237.3+t}\right) \\
u_{9}=u_{h}\left(\frac{h_{1}}{h}\right)^{1 / 7} \\
E_{L}=K_{M}\left(e_{w}-e_{a}\right)\left(1+\frac{u_{9}}{16}\right) \\
E_{a}=0.35\left(1+\frac{u_{2}}{160}\right)\left(e_{w}-e_{a}\right) \\
f_{p}=f_{c}+\left(f_{0}-f_{c}\right) e^{-K_{h} t} \\
\mathrm{PET}=\frac{A H_{n}+E_{a \gamma}}{A+\gamma} \\
P_{r, n}={ }^{n} C_{r} P^{r} q^{n-r} \\
=\frac{n!}{(n-r)!r!} P^{r} q^{n-r}
\end{gathered}
$$

## Appendix

Normal Distribution Table


| Normal Deviate z | . 00 | . 01 | . 02 | . 03 | . 04 | . 05 | . 06 | . 07 | . 08 | . 09 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| -4.0 | . 0000 | . 0000 | . 0000 | . 0000 | . 0000 | . 0000 | . 0000 | . 0000 | . 0000 | . 0000 |
| -3.9 | . 0000 | . 0000 | . 0000 | . 0000 | . 0000 | . 0000 | . 0000 | . 0000 | . 0000 | . 0000 |
| -3.8 | . 0000 | . 0000 | . 0000 | . 0000 | . 0000 | . 0000 | . 0000 | . 0000 | . 0000 | . 0000 |
| -3.7 | . 0001 | . 0001 | . 0000 | . 0000 | . 0000 | . 0000 | . 0000 | . 0000 | . 0000 | . 0000 |
| -3.6 | . 0002 | . 0002 | . 0001 | . 0001 | . 0001 | . 0001 | . 0001 | . 0001 | . 0001 | . 0001 |
| -3.5 | . 0002 | . 0002 | . 0002 | . 0002 | . 0002 | . 0002 | . 0002 | . 0002 | . 0002 | . 0002 |
| -3.4 | . 0003 | . 0003 | . 0003 | . 0003 | . 0003 | . 0003 | . 0003 | . 0003 | . 00003 | . 0002 |
| -3.3 | . 0005 | . 0005 | . 0005 | . 0004 | . 0004 | . 0004 | . 0004 | . 0004 | . 0004 | . 0003 |
| -3.2 | . 0007 | . 00007 | . 0006 | . 0006 | . 0006 | . 0006 | . 0006 | . 0005 | . 0005 | . 0005 |
| -3.1 | . 0010 | . 0009 | . 0009 | . 0009 | . 0008 | . 0008 | . 0008 | . 0008 | . 0007 | . 0007 |
| -3.0 | . 0013 | . 0013 | . 0013 | . 0012 | . 0012 | . 0011 | . 0011 | . 0011 | . 0010 | . 0010 |
| -2.9 | . 0019 | . 0018 | . 0018 | . 0017 | . 0016 | . 0016 | . 0015 | . 0015 | . 0014 | . 0014 |
| -2.8 | . 0026 | . 0025 | . 0024 | . 0023 | . 0023 | . 0022 | . 0021 | . 0021 | . 0020 | . 0019 |
| -2.7 | . 0035 | . 0034 | . 0033 | . 0032 | . 0031 | . 0030 | . 0029 | . 0028 | . 0027 | . 0026 |
| -2.6 | . 0047 | . 0045 | . 0044 | . 0043 | . 0041 | . 0040 | . 0039 | . 0038 | . 0037 | . 0036 |
| -2.5 | . 0062 | . 0060 | . 0059 | . 0057 | . 0055 | . 0054 | . 0052 | . 0051 | . 0049 | . 0048 |
| -2.4 | . 0082 | . 0080 | . 0078 | . 0075 | . 0073 | . 0071 | . 0069 | . 0068 | . 0066 | . 0064 |
| -2.3 | . 0107 | . 0104 | . 0102 | . 0099 | . 0096 | . 0094 | . 0091 | . 0089 | . 0087 | . 0084 |
| -2.2 | . 0139 | . 0136 | . 0132 | . 0129 | . 0125 | . 0122 | . 0119 | . 0116 | . 0113 | . 0110 |
| -2.1 | . 0179 | . 0174 | . 0170 | . 0166 | . 0162 | . 0158 | . 0154 | . 0150 | . 0146 | . 0143 |
| -2.0 | . 0228 | . 0222 | . 0217 | . 0212 | . 0207 | . 0202 | . 0197 | . 0192 | . 0188 | . 0183 |
| -1.9 | . 0287 | . 0281 | . 0274 | . 0268 | . 0262 | . 0256 | . 0250 | . 0244 | . 0239 | . 0233 |
| -1.8 | . 0359 | . 0351 | . 0344 | . 0336 | . 0329 | . 0322 | . 0314 | . 0307 | . 0301 | . 0294 |
| -1.7 | . 0446 | . 0436 | . 0427 | . 0418 | . 0409 | . 0401 | . 0392 | . 0384 | . 0375 | . 0367 |
| -1.6 | . 0548 | . 0537 | . 0526 | . 0516 | . 0505 | . 0495 | . 0485 | . 0475 | . 0465 | . 0455 |
| -1.5 | . 0668 | . 0655 | . 0643 | . 0630 | . 0618 | . 0606 | . 0594 | . 0582 | . 0571 | . 0559 |
| -1.4 | . 0808 | . 0793 | . 0778 | . 0764 | . 0749 | . 0735 | . 0721 | . 0708 | . 0694 | . 0681 |
| -1.3 | . 0968 | . 0951 | . 0934 | . 0918 | . 0901 | . 0885 | . 0869 | . 0853 | . 0838 | . 0823 |
| -1.2 | . 1151 | . 1131 | . 1112 | . 1093 | . 1075 | . 1056 | . 1038 | . 1020 | . 1003 | . 0985 |
| -1.1 | . 1357 | . 1335 | . 1314 | . 1292 | . 1271 | . 1251 | . 1230 | . 1210 | . 1190 | . 1170 |
| -1.0 | . 1587 | . 1562 | . 1539 | . 1515 | . 1492 | . 1469 | . 1446 | . 1423 | . 1401 | . 1379 |
| -. 9 | . 1841 | . 1814 | . 1788 | . 1762 | . 1736 | . 1711 | . 1685 | . 1660 | . 1635 | . 1611 |
| -. 8 | . 2119 | . 2090 | . 2061 | . 2033 | . 2005 | . 1977 | . 1949 | . 1922 | . 1894 | . 1867 |
| -. 7 | . 2420 | . 2389 | . 2358 | . 2327 | . 2296 | . 2266 | . 2236 | . 2206 | . 2177 | . 2148 |
| -. 6 | . 2743 | . 2709 | . 2676 | . 2643 | . 2611 | . 2578 | . 2546 | . 2514 | . 2483 | . 2451 |
| -. 5 | . 3085 | . 3050 | . 3015 | . 2981 | . 2946 | . 2912 | . 2877 | . 2843 | . 2810 | . 2776 |
| -. 4 | . 3446 | . 3409 | . 3372 | . 3336 | . 3300 | . 3264 | . 3228 | . 3192 | . 3156 | . 3121 |
| -. 3 | . 3821 | . 3783 | . 3745 | . 3707 | . 3669 | . 3632 | . 3594 | . 3557 | . 3520 | . 3483 |


| Z | 0.00 | 0.01 | 0.02 | 0.03 | 0.04 | 0.05 | 0.06 | 0.07 | 0.08 | 0.09 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 0.0 | 0.0000 | 0.0040 | 0.0080 | 0.0120 | 0.0160 | 0.0199 | 0.0239 | 0.0279 | 0.0319 | 0.0359 |
| 0.1 | 0.0398 | 0.0438 | 0.0478 | 0.0517 | 0.0557 | 0.0596 | 0.0636 | 0.0675 | 0.0714 | 0.0753 |
| 0.2 | 0.0793 | 0.0832 | 0.0871 | 0.0910 | 0.0948 | 0.0987 | 0.1026 | 0.1064 | 0.1103 | 0.1141 |
| 0.3 | 0.1179 | 0.1217 | 0.1255 | 0.1293 | 0.1331 | 0.1368 | 0.1406 | 0.1443 | 0.1480 | 0.1517 |
| 0.4 | 0.1554 | 0.1591 | 0.1628 | 0.1664 | 0.1700 | 0.1736 | 0.1772 | 0.1808 | 0.1844 | 0.1879 |
| 0.5 | 0.1915 | 0.1950 | 0.1985 | 0.2019 | 0.2054 | 0.2088 | 0.2123 | 0.2157 | 0.2190 | 0.2224 |
| 0.6 | 0.2257 | 0.2291 | 0.2324 | 0.2357 | 0.2389 | 0.2422 | 0.2454 | 0.2486 | 0.2517 | 0.2549 |
| 0.7 | 0.2580 | 0.2611 | 0.2642 | 0.2673 | 0.2704 | 0.2734 | 0.2764 | 0.2794 | 0.2823 | 0.2852 |
| 0.8 | 0.2881 | 0.2910 | 0.2939 | 0.2967 | 0.2995 | 0.3023 | 0.3051 | 0.3078 | 0.3106 | 0.3133 |
| 0.9 | 0.3159 | 0.3186 | 0.3212 | 0.3238 | 0.3264 | 0.3289 | 0.3315 | 0.3340 | 0.3365 | 0.3389 |
| 1.0 | 0.3413 | 0.3438 | 0.3461 | 0.3485 | 0.3508 | 0.3531 | 0.3554 | 0.3577 | 0.3599 | 0.3621 |
| 1.1 | 0.3643 | 0.3665 | 0.3686 | 0.3708 | 0.3729 | 0.3749 | 0.3770 | 0.3790 | 0.3810 | 0.3830 |
| 1.2 | 0.3849 | 0.3869 | 0.3888 | 0.3907 | 0.3925 | 0.3944 | 0.3962 | 0.3980 | 0.3997 | 0.4015 |
| 1.3 | 0.4032 | 0.4049 | 0.4066 | 0.4082 | 0.4099 | 0.4115 | 0.4131 | 0.4147 | 0.4162 | 0.4177 |
| 1.4 | 0.4192 | 0.4207 | 0.4222 | 0.4236 | 0.4251 | 0.4265 | 0.4279 | 0.4292 | 0.4306 | 0.4319 |
| 1.5 | 0.4332 | 0.4345 | 0.4357 | 0.4370 | 0.4382 | 0.4394 | 0.4406 | 0.4418 | 0.4429 | 0.4441 |
| 1.6 | 0.4452 | 0.4463 | 0.4474 | 0.4484 | 0.4495 | 0.4505 | 0.4515 | 0.4525 | 0.4535 | 0.4545 |
| 1.7 | 0.4554 | 0.4564 | 0.4573 | 0.4582 | 0.4591 | 0.4599 | 0.4608 | 0.4616 | 0.4625 | 0.4633 |
| 1.8 | 0.4641 | 0.4649 | 0.4656 | 0.4664 | 0.4671 | 0.4678 | 0.4686 | 0.4693 | 0.4699 | 0.4706 |
| 1.9 | 0.4713 | 0.4719 | 0.4726 | 0.4732 | 0.4738 | 0.4744 | 0.4750 | 0.4756 | 0.4761 | 0.4767 |
| 2.0 | 0.4772 | 0.4778 | 0.4783 | 0.4788 | 0.4793 | 0.4798 | 0.4803 | 0.4808 | 0.4812 | 0.4817 |
| 2.1 | 0.4821 | 0.4826 | 0.4830 | 0.4834 | 0.4838 | 0.4842 | 0.4846 | 0.4850 | 0.4854 | 0.4857 |
| 2.2 | 0.4861 | 0.4864 | 0.4868 | 0.4871 | 0.4875 | 0.4878 | 0.4881 | 0.4884 | 0.4887 | 0.4890 |
| 2.3 | 0.4893 | 0.4896 | 0.4898 | 0.4901 | 0.4904 | 0.4906 | 0.4909 | 0.4911 | 0.4913 | 0.4916 |
| 2.4 | 0.4918 | 0.4920 | 0.4922 | 0.4925 | 0.4927 | 0.4929 | 0.4931 | 0.4932 | 0.4934 | 0.4936 |
| 2.5 | 0.4938 | 0.4940 | 0.4941 | 0.4943 | 0.4945 | 0.4946 | 0.4948 | 0.4949 | 0.4951 | 0.4952 |
| 2.6 | 0.4953 | 0.4955 | 0.4956 | 0.4957 | 0.4959 | 0.4960 | 0.4961 | 0.4962 | 0.4963 | 0.4964 |
| 2.7 | 0.4965 | 0.4966 | 0.4967 | 0.4968 | 0.4969 | 0.4970 | 0.4971 | 0.4972 | 0.4973 | 0.4974 |
| 2.8 | 0.4974 | 0.4975 | 0.4976 | 0.4977 | 0.4977 | 0.4978 | 0.4979 | 0.4979 | 0.4980 | 0.4981 |
| 2.9 | 0.4981 | 0.4982 | 0.4982 | 0.4983 | 0.4984 | 0.4984 | 0.4985 | 0.4985 | 0.4986 | 0.4986 |
| 3.0 | 0.4987 | 0.4987 | 0.4987 | 0.4988 | 0.4988 | 0.4989 | 0.4989 | 0.4989 | 0.4990 | 0.4990 |
| 3.1 | 0.4990 | 0.4991 | 0.4991 | 0.4991 | 0.4992 | 0.4992 | 0.4992 | 0.4992 | 0.4993 | 0.4993 |
| 3.2 | 0.4993 | 0.4993 | 0.4994 | 0.4994 | 0.4994 | 0.4994 | 0.4994 | 0.4995 | 0.4995 | 0.4995 |
| 3.3 | 0.4995 | 0.4995 | 0.4995 | 0.4996 | 0.4996 | 0.4996 | 0.4996 | 0.4996 | 0.4996 | 0.4997 |
| 3.4 | 0.4997 | 0.4997 | 0.4997 | 0.4997 | 0.4997 | 0.4997 | 0.4997 | 0.4997 | 0.4997 | 0.4998 |

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