

**PART A / BAHAGIAN A**

- (1). (a). The results of a sieve analysis on a soil sample were as follows:  
*Keputusan analisa saringan suatu tanah adalah seperti berikut:*

Sieve size (mm)	Mass Retained (g)
10.00	0.0
6.00	5.5
2.00	25.7
1.00	23.1
0.60	22.0
0.30	17.3
0.15	12.7
0.063	6.9

In addition, 2.30 gram was found to pass the 63  $\mu\text{m}$  sieve size. Using the particle size distribution chart provided (Appendix 1), **plot** the particle size distribution curve. **Determine** the uniformity coefficient ( $C_u$ ) and **classify** the soil sample based on the chart provided.

*Selain itu, sebanyak 2.30 gram didapati melepasi saringan saiz 63  $\mu\text{m}$ . Berdasarkan carta taburan saiz zarah yang disediakan (Lampiran 1), **plotkan** lengkungan taburan saiz zarah. **Tentukan** pekali keseragaman ( $C_u$ ) dan **kelaskan** sampel tanah ini menggunakan carta yang dibekalkan.*

(10 marks/markah)

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(b). In a constant head permeameter test the following results were obtained:

- Duration of test = 5.0 min
- Quantity of water collected/ = 900 ml
- Head difference in manometer = 50 mm
- Distance between manometer tapings/ = 100 mm
- Diameter of test sample/ = 120 mm

**Determine** the coefficient of permeability in m/s.

*Maklumat berikut diperolehi daripada ujian kebolehtelapan malar:*

- *Tempoh ujikaji* = 5.0 min
- *Kuantiti air diperolehi* = 900 ml
- *Perbezaan turus di dalam manometer* = 50 mm
- *Jarak antara* = 100 mm
- *Diameter sampel ujikaji* = 120 mm

**Tentukan** pekali kebolehtelapan di dalam m/s.

(4 marks/markah)

(c). **Distinguish** between fine sand, silt and clay.

**Bezakan** di antara pasir halus, kelodak dan lempung/tanah liat.

(6 marks/markah)

(2). (a). Plot the poles of the following discontinuities, using Appendix 2:

- (i). dip  $57^\circ$  towards  $270^\circ$
- (ii). dip  $64^\circ$  towards  $284^\circ$
- (iii). dip  $45^\circ$  towards  $280^\circ$
- (iv). dip  $55^\circ$  towards  $272^\circ$

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- (v). dip  $60^\circ$  towards  $090^\circ$
- (vi). dip  $53^\circ$  towards  $275^\circ$
- (vii). dip  $13^\circ$  towards  $170^\circ$
- (viii). dip  $53^\circ$  towards  $275^\circ$

Determine the estimated general dip and trend of these structures.

*Plot kutub-kutub ketakselajaran berikut, menggunakan Appendix 2:*

- (i). *miring  $57^\circ$  ke arah  $270^\circ$*
- (ii). *miring  $64^\circ$  ke arah  $284^\circ$*
- (iii). *miring  $45^\circ$  ke arah  $280^\circ$*
- (iv). *miring  $55^\circ$  ke arah  $272^\circ$*
- (v). *miring  $60^\circ$  ke arah  $090^\circ$*
- (vi). *miring  $53^\circ$  ke arah  $275^\circ$*
- (vii). *miring  $13^\circ$  ke arah  $170^\circ$*
- (viii). *miring  $53^\circ$  ke arah  $275^\circ$*

*Tentukan anggaran kemiringan am dan tren struktur-struktur ini.*

*(10 marks/markah)*

- (b). Based on the obtained estimated mean pole previously (a), determine the angle between two mean poles of second joint set at a dip direction of  $230^\circ$ , and a dip of  $42^\circ$ . Determine the plunge and trend of the line of the intersection between the two joint sets.

*Berdasarkan anggaran min kutub yang diperolehi sebelumnya (a), tentukan sudut di antara dua set min kutub bersama set kekar kedua pada arah kemiringan  $230^\circ$ , dan miring sebanyak  $42^\circ$ .*

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*Tentukan tunjaman dan trend garis persilangan di antara dua set kekar.*

*(4 marks/markah)*

- (c). The stress in a rock mass has been measured by the hydraulic fracturing technique. Two tests were conducted in a vertical borehole: one test at a depth of 500 m; and the other test at a depth of 1000 m. The results were as follows:

*Tegasan dalam jisim batuan telah diukur dengan teknik keretakan hidraulik. Dua ujian telah dijalankan dalam lubang gerudi menegak: satu ujian pada kedalaman 500 m; dan ujian yang lain pada kedalaman 1000 m. Keputusan adalah seperti berikut:*

Depth, m	Breakdown Pressure, Mpa	Shut-in Pressure, Mpa
500	14	8
1000	24.5	16

Given that the tensile strength of the rock is 10 Mpa, estimate and list the values of  $\sigma_1$ ,  $\sigma_2$  and  $\sigma_3$  at the two depths. Discuss all the assumptions you have to make in order to produce these estimations. State whether the two sets of results are consistent with each other and justify the reasons for the statement.

*Memandangkan kekuatan tegangan daripada batu adalah 10 Mpa, anggarkan dan senaraikan nilai-nilai  $\sigma_1$ ,  $\sigma_2$  dan  $\sigma_3$  di kedua-dua kedalaman. Bincangkan semua andaian yang anda perlu buat untuk menghasilkan anggaran ini. Nyatakan sama ada dua set keputusan adalah konsisten dengan satu sama lain dan wajarkan sebab-sebab bagi kenyataan itu.*

*(6 marks/markah)*

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**PART B / BAHAGIAN B**

- (3). (a). Briefly **discuss** the best practice for the ground investigation of an existing slope, or the ground onto which a slope is to be built.

***Bincangkan** secara ringkas langkah praktikal terbaik untuk menjalankan penyiasatan lapangan terhadap cerun siap dibina ataupun lapangan yang akan dibina cerun.*

(5 marks/ markah)

- (b). Figure 1 as follows, gives details of an embankment made of cohesive soil with  $\phi_u = 0$  and  $C_u = 23 \text{ kN/m}^2$ . The unit weight of the soil is  $19 \text{ kN/m}^3$ . The weight of sliding sector is 398 kN acting at the eccentricity of 5.7 m from the center of rotation.

Note: In both cases assume that no tension crack develops.

*Rajah 1 menunjukkan benteng yang diperbuat daripada tanah berjelekit dengan  $\phi_u = 0$  and  $C_u = 23 \text{ kN/m}^2$ . Jisim unit tanah adalah  $19 \text{ kN/m}^3$ . Berat sektor gelinciran adalah 398 kN bertindak terhadap keeksentrikan 5.7 m daripada pusat putaran.*

*Nota: Anggapkan tiada keretakan di dalam kedua-dua situasi.*

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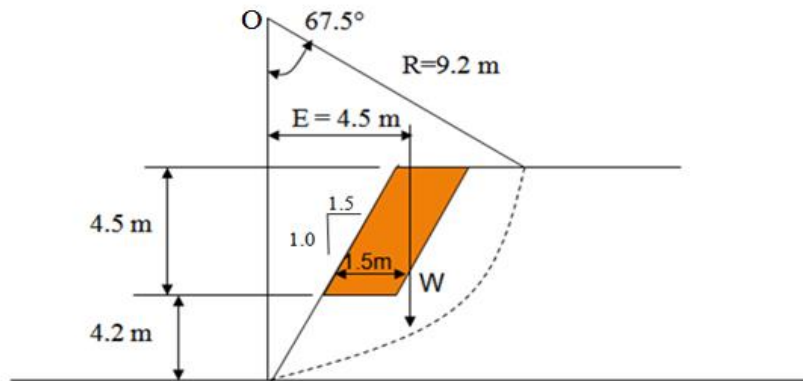


Figure 1 / Rajah 1

- (i). For the trial circle shown, **determine** the factor of safety against sliding.

*Berdasarkan bulatan percubaan seperti yang ditunjukkan, tentukan faktor keselamatan terhadap gelinciran.*

(4 marks/markah)

- (ii) **Analyse** the new factor of safety if the shaded portion of the embankment were removed.

*Analisa faktor keselamatan yang baharu sekiranya bahagian yang berlorek dikeluarkan.*

(8 marks/markah)

- (iii). **Calculate** the percentage increment between the factor of safety for the case of 3b(i) and 3b(ii) above. Provide a **comment** on this.

*Kirakan peningkatan peratusan faktor keselamatan antara kedua-dua situasi 3b(i) dan 3b(ii) di atas. Berikan komen berkenaan perkara ini.*

(3 marks/ markah)

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(4). (a).

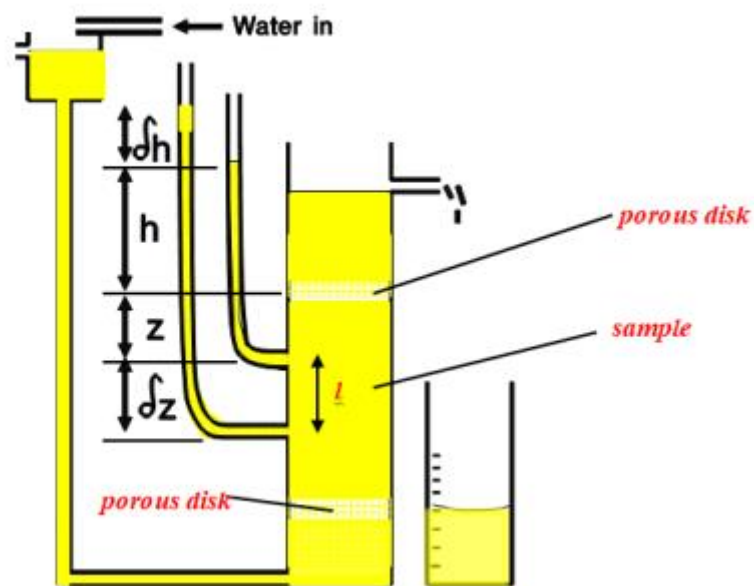


Figure 2: Constant head permeameter test

*Rajah 2: Ujian kebolehtelapan malar*

Water flows through the soil sample under a head which is kept constant by means of the overflow arrangement as shown in Figure 2. The head loss,  $h$ , between two points along the length of the sample, distance,  $l$ , apart is measured by means of a manometer. By the means of the Darcy's law,  $q = Aki$  and assuming that the area of sample is  $A$ , **derive** the expression of coefficient of permeability,  $k$ .

*Aliran air melalui sampel tanah secara turus malar dengan aturan aliran limpah adalah seperti yang ditunjukkan di dalam Rajah 2. Kehilangan turus malar,  $h$ , di antara dua titik di sepanjang sampel yang panjangnya,  $l$ , diukur menggunakan manometer. Dengan menggunakan Hukum Darcy,  $q=Aki$  dan mengandaikan luas sampel adalah  $A$ , **terbitkan** persamaan pekali kebolehtelapan,  $k$ .*

(3 marks/markah)

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- (b). A granular soil has a saturated unit weight of  $19.5 \text{ kN/m}^3$  and angle of shearing resistance of  $32^\circ$ . A slope is to be made of this soil. If the factor of safety is to be 1.4, **determine** the safe angle of the slope it:

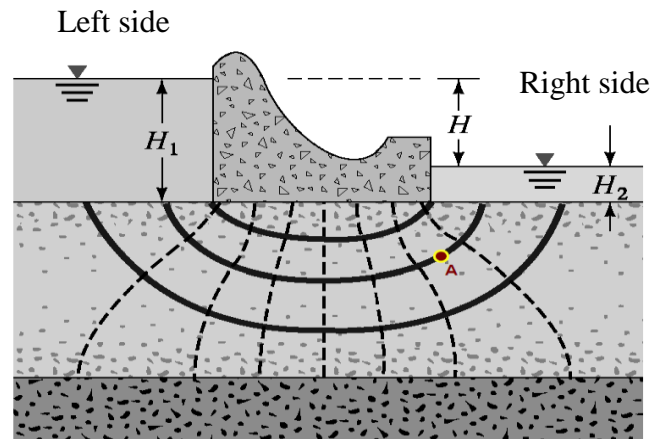
*Satu tanah granular mempunyai berat unit tepu  $19.5 \text{ kN/m}^3$  dan sudut rintangan ricih  $32^\circ$ . Sebuah cerun dibina menggunakan tanah tersebut. Andaikan faktor keselamatan yang diperlukan adalah 1.4, **tentukan** sudut selamat bagi cerun tersebut jika:*

- (i). The slope is in dry condition  
*Cerun dalam keadaan kering.*  
(2 marks/markah)

- (ii). If seepage occurs at and parallel to the surface of slope.  
*Jika resipan muncul pada dan selari dengan permukaan cerun*  
(2 marks/markah)

- (c). **Explain** what is meant by the flow net.  
**Jelaskan** maksud jaringan aliran.  
(3 marks/markah)

(d).



**Figure 3:** Flow Net underneath a Gravity Dam

(Note: Figure is Not To Scale).

Figure 3 shows the flownet of a dam to be constructed. A piezometer (i.e. standpipe) is installed at Point A. The following are some dimensions of the flow net:

*Rajah 3 menunjukkan jaringan aliran empangan yang akan dibina. Sebuah piezometer (paip) dipasang pada Titik A. Berikut adalah sebahagian dimensi jaringan aliran tersebut:*

- Dam Length / Panjang empangan = 175 m.
- $H = 25$  m
- $H_1 = 35$  m
- The depth of Point A below the dam  
*Kedalaman Titik A daripada aras empangan = 16 m.*

(i). **Determine** the depth of water drop in the piezometer on the left side of the dam.

**Tentukan** kedalaman air yang jatuh di dalam piezometer di sebelah kiri empangan.

(4 marks/markah)

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- (ii). **Determine** the rise of the water level in the piezometer relative to the water level on the right side of the dam.

*Tentukan kenaikan paras air di dalam piezometer berkadaran dengan paras air di sebelah kanan empangan.*

(2 marks/markah)

- (iii). **Calculate** the pore pressure at Point A.

*Kirakan tekanan liang di Titik A.*

(Given unit weigh of water is  $9.81 \text{ kg/m}^3$ )

(Diberi berat unit bagi air adalah  $9.81 \text{ kg/m}^3$ )

(4 marks/markah)

- (5). (a). A new bench of an open pit mine is going to cut through a granite hill. The rock slope of the bench is going to dip towards  $300^\circ$ . The rock has two sets of joint as follow:

Set 1 dips  $52^\circ$  towards  $258^\circ$

Set 2 dips  $60^\circ$  towards  $333^\circ$

The angles of the bench cut are being considered at the following angles:

(i).  $60^\circ$

(ii).  $52^\circ$

(iii).  $40^\circ$

Discuss on the type of failures (if any) that may occur on each proposed slope. What is the steepest slope that you would recommend and why? Attach all tracing papers used with your answers.

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*Sebuah jalan raya akan dibina dan akan memotong sebahagian daripada sebuah bukit granit. Cerun di tepi jalan raya ini miring ke arah 300°. Batuan yang akan dipotong ini terdapat dua kekar iaitu :*

*Set 1 miring 52° ke arah 258°*

*Set 2 miring 60° ke arah 333°*

*Sudut muka cerun potongan ini sedang dipertimbangkan seperti berikut:*

- (i). 60°*
- (ii). 52°*
- (iii). 40°*

*Bincangkan mengenai jenis kegagalan (jika ada) yang mungkin terjadi pada setiap sudut muka cerun yang dicadangkan. Apakah sudut muka cerun yang paling curam yang anda cadangkan dan nyatakan mengapa? Jawapan anda hendaklah disertakan dengan kertas surih yang anda gunakan.*

*(12 marks/markah)*

(b). Please define and discuss the following:

- (i). Rock mass
- (ii). *In-situ* stress
- (iii). Anisotropy
- (iv). Uniaxial Compressive Strength

*Takrif dan terangkan mengenai perkara-perkara berikut:*

- (i). Jisim batuan*
- (ii). Tegasan in-situ*

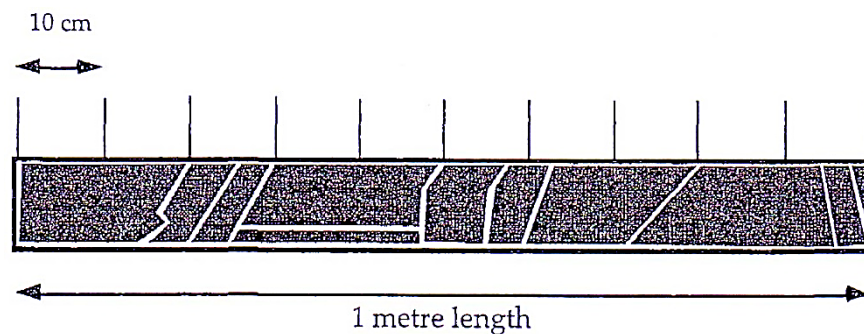
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- (iii). *Ketakselajaran*  
 (iv). *Kekuatan mampatan unipaksi*

(4 marks/markah)

- (b). For the following sketch, showing a 1 meter long section of recovered drill core, discuss on the numerical estimation of Rock Quality Designation (RQD) for the core length and what rock quality would be associated with this rock. Provide a brief description of the RQD classification technique.

*Lakaran di bawah menunjukkan batuan dari lubang gerudi sepanjang 1 meter, bincangk nilai jangkaan RQD bagi lubang gerudi tersebut dan apakah kualiti batuan yang boleh dikaitkan dengan batuan ini. Sertakan penjelasan secara ringkas berkaitan teknik klasifikasi RQD*



(4 marks/markah)

- (6). (a). A 30 m high face slope has been excavated northward into a rock mass. Extensive geotechnical drilling programme has revealed the following information:

Plane A      dips  $32^\circ$  towards  $158^\circ$ ,  $\phi$  of 35  
 Plane B      dips  $60^\circ$  towards  $333^\circ$ ,  $\phi$  of 20

...14/-

The intersection of the two planes will daylight on the slope face, and the water table has been identified as located a few meters below the proposed excavation. At the opposite site of the slope, plane A formed a wedge intersection with Plane C having a dip of  $40^\circ$ , a dip direction of  $012^\circ$  and a  $\phi$  of  $31^\circ$ ; Plane C is also planar, smooth and has zero cohesion. Very shortly after this intersection daylighted out of its slope, the wedge failed. Using accompanying design charts (Appendix 3), determine the factor of safety of the wedge formed by planes A, B and C.

*Satu muka cerun setinggi 30 m telah dikorek ke arah utara di dalam jisim suatu batuan. Program penggerudian geoteknikal telah menghasilkan maklumat yang berikut:*

*Satah A        miring  $32^\circ$  ke arah  $158^\circ$ , dengan  $\phi$  35*

*Satah B        miring  $60^\circ$  ke arah  $333^\circ$ , dengan  $\phi$  20*

*Persilangan kedua-dua satah akan bercahaya siang di muka cerun, dan air di dalam tanah telah dikesan berada beberapa meter di bawah aras tempat penggalian yang dicadangkan. Di kawasan bertentangan cerun tersebut, satah A membentuk persilangan baji dengan satah C yang mempunyai kemiringan  $40^\circ$ , arah miring  $012^\circ$  dan  $\phi$  bernilai  $31^\circ$ ; sesar ini juga berbentuk planar dan licin dan mempunyai kejeleketan kosong. Baji ini gagal tidak lama setelah persilangan terdedah di muka cerun. Dengan menggunakan carta yang disediakan (Lampiran 3), tentukan faktor keselamatan baji yang dibentuk oleh satah-satah A, B dan C.*

(10 marks/markah)

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- (b). Rock stabilisation measures can be characterised on the basis of them offering either reinforcement or support to the rock.

*Langkah-langkah penstabilan batu boleh dicirikan atas dasar mereka yang menawarkan sama ada tetulang atau sokongan kepada batu.*

- (i). Commencing with a statement of the purpose of rock stabilisation, discuss the fundamental behavioural differences between rock reinforcement and rock support, paying particular attention to how the differences are affected by the presence of discontinuous and continuous rock masses.

*Bermula dengan pernyataan tujuan penstabilan batuan, bincangkan perbezaan perilaku asas antara penguatan batu dan sokongan batu, dengan memberi perhatian khusus tentang bagaimana perbezaannya dipengaruhi oleh kehadiran ketakselajaran dan jisim batuan berterusan.*

(5 marks/markah)

- (ii). With reference to the ground response curve, discuss how support requirements are affected by the excavation method, the degree of fracturing of the rock mass, and the time of emplacement of the support.

*Dengan merujuk kepada keluk tindak balas tanah, bincangkan bagaimana keperluan sokongan dipengaruhi oleh kaedah penggalian, darjah rekahan jisim batuan, dan masa pemisahan sokongan.*

(5 marks/markah)

...16/-

- (7). (a). A 5.5 m diameter tunnel is to be driven through a sequence of brecciated schist, phyllite and gneiss rock at a minimum depth of 1,200 m. The rock strata dips towards the east and being cut by two major discontinuities orientated at N285° and N250°. The bedding dips between 15° and 20°, the faults dip at 70° and 60° respectively. The faults are smooth planar to slickenside planar, moderately narrow, highly weathered and healed with recrystalline quartz infilling, but overall the rock is described as 'highly weathered and weak'. The groundwater level is about 10 m below the ground surface and recorded at < 1L/min. The rock mass is easily broken by normal blow of hammer with the average UCS of is 45 MPa, and the horizontal stress is about 3.4 MPa.

Use the RMR and Q system (Appendix 4 & 5) to classify this rock and assess the stability of the tunnel as its being driven from northeast to southwest.

*Terowong dengan diameter 5.5 m akan di bina di dalam urutan batuan syis berbreksia, filit dan gneis pada kedalaman tidak kurang dari 1,200 m. Strata batuan ini miring ke arah timur, dan dipotong oleh dua ketakselajaran utama yang berorientasi pada arah N285° and N250°. Perlapisan batuan miring di antara 15° dan 20°, sesar miring masing-masing pada 70 ° dan 60 °. Sesar adalah satah licin kepada upaman, sederhana sempit amat terluluhawa dan ditutupi oleh kuarza berpenghabluran semula, tetapi secara keseluruhan batuan diperihalkan sebagai 'sangat terluluhawa dan lemah'. Air bawah tanah ditemui pada 10 m di bawah permukaan tanah dan direkod pada <1L/min. Jisim batuan mudah pecah dengan pukulan biasa oleh tukul dengan tegasan pada batuan utuh pada 45 MPa dan tekanan mendatar pada 3.4 MPa.*

...17/-

*Gunakan sistem klasifikasi RMR dan Q (Lampiran 4 & 5) untuk batuan ini, dan buat penilaian kestabilan terowong ini sekiranya proses pengorekan dilakukan dari utara timur ke selatan barat.*

*(20 marks/markah)*

**-oooOooo-**