EFFICIENCY OF USING STEEL END CAPS IN IMPROVING THE POST-FIRE FLEXURAL BEHAVIOR OF FRP REINFORCED CONCRETE BEAMS

RAMI J. A. HAMAD

UNIVERSITI SAINS MALAYSIA 2017

EFFICIENCY OF USING STEEL END CAPS IN IMPROVING THE POST-FIRE FLEXURAL BEHAVIOR OF FRP REINFORCED CONCRETE BEAMS

by

RAMI J. A. HAMAD

This submitted in fulfilment of the requirements for the degree of Doctor of Philosophy

بسم الله الرحمن الرحيم

(يَرْفَع اللَّهُ الَّذِينَ آمَنُوا مِنكُمْ وَالَّذِينَ أُوتُوا الْعِلْمَ دَرَجَاتٍ وَاللَّهُ بِمَا تَعْمَلُونَ خَبِيرٌ)

سورة المجادلة - 11

In The Name of Allah, the Most Beneficent, the Most Merciful

(Allah will raise those who have believed among you and those who were given knowledge, by degrees. And Allah is acquainted with what you do)

Surat Al-Mujadala -11

DEDICATION

To my mother, father, sisters, and brothers,

To my beloved wife, sons and daughter,

To all my friends in Palestine, Malaysia, Jordan and UAE,

To everyone who was there when I needed support,

To all those who wished me luck,

I dedicate this work with all my love.

ACKNOWLEDGMENT

First and foremost, I have to thank my parents for their love and support throughout my life.

All my gratitude goes to my main supervisor Prof. Dr. Megat Azmi Megat Johari and my Co-supervisor Prof. Rami Haddad for their guidance, support and patience.

Sincere thanks to all technicians in the engineering laboratories and workshops for their continuous support.

Exceptional thanks to all academic staff, administrative staff and my colleagues in the School of Civil Engineering who were of great help and support.

Last, but not least, I express my warm thanks to my wife for her moral and spiritual support, quiet patience and unwavering love.

TABLE OF CONTENTS

		Page
AC	KNOWLEDGMENT	ii
TAI	BLE OF CONTENTS	iii
LIS	T OF TABLES	ix
LIS	T OF FIGURES	xii
LIS	T OF ABBREVIATIONS	xix
LIS	T OF SYMBOLS	xxii
ABS	STRAK	xxviii
ABS	STRACT	xxx
CH	APTER ONE: INTRODUCTION	
1.1	Preface	1
1.2	Problem statement	3
1.3	Research objectives	5
1.4	Scope of work	5
1.5	Thesis structure	8
CH	APTER TWO: LITERATURE REVIEW	
2.1	Introduction	9
2.2	Fiber Reinforced Polymer (FRP) composites	9
2.3	FRP composites in concrete construction	12
	2.3.1 Bond between FRP bars and concrete	13
	2.3.2 Performance of concrete beams with FRP internal reinforcement	19
	bars	
2.4	Performance of FRP composites exposed to high temperatures	26

	2.4.1	Introduction	26
	2.4.2	Effect of fire on thermal properties of FRP	28
	2.4.3	Effect of fire on mechanical properties of FRP	30
	2.4.4	Effect of fire on deformation properties of FRP	35
	2.4.5	Effect of fire on bond strength between FRP composites and concrete	36
	2.4.6	Fire tests on FRP-RC structures	41
2.5	Concr	ete under high temperature	51
2.6	Summ	nary	59
CII	A DODGE	A THIRTY AND ALOW	
СН	APTE	R THREE: METHODOLOY	
3.1	Introd	uction	60
3.2	Mater	ials	61
	3.2.1	Reinforcement bars	61
		3.2.1(a) FRP bars	61
		3.2.1(b) Reinforcing steel bars	61
	3.2.2	Concrete	62
		3.2.2(a) Water	63
		3.2.2(b) Cement	63
		3.2.2(c) Coarse aggregate	63
		3.2.2(d) Fine aggregate	63
	3.2.3	High tensile strength epoxy	64
	3.2.4	High temperature resistant epoxy adhesive	64
	3.2.5	Stainless steel end cap	65
	3.2.6	High temperature thermal insulation coating	66
3.3	Testin	g program	67

3.4	Specia	imens preparation 70		
	3.4.1	Preparation of tensile testing specimens	70	
	3.4.2	Preparation of pull-out testing specimens	74	
	3.4.3	RC beams preparation	76	
3.5	Mixin	g, casting, and curing of different specimens	80	
3.6	Heatin	ng processes	83	
	3.6.1	Heating of pull-out specimens	83	
	3.6.2	Heating of beam specimens	85	
	3.6.3	Heating of concrete standard specimens	86	
3.7	Mech	anical testing	87	
	3.7.1	Tension test of FRP/steel bars	87	
	3.7.2	Pull-out test	89	
	3.7.3	Flexural response test	91	
	3.7.4	Compression and splitting test of concrete	92	
3.8	Theor	etical work	93	
3.7	Sumn	nary	94	
CH	APTEI	R FOUR: RESULTS AND DISCUSSION		
4.1	Introd	uction	95	
4.2	Temp	erature profile for RC beams	95	
4.3	Post-l	neating mechanical properties of reinforcement bars	97	
	4.3.1	Characteristics of FRP bars at different temperature exposures	97	
	4.3.2	Characteristics of steel bars at different temperature exposures	104	
4.4	Resid	ual mechanical properties of concrete	106	
	4.4.1	Compressive strength	106	

	4.4.2	Splitting	tensile strength	107
4.5	Bond	strength te	est results	108
	4.5.1	Bond bet	tween FRP/steel bars and concrete at ambient ure	108
	4.5.2	Post-heat	ting bond behavior between FRP/Steel bars and	109
		4.5.2(a)	Introduction	109
		4.5.2(b)	Bond stress versus slip relationship	110
		4.5.2(c)	Characteristics of bond stress versus slip curves	112
		4.5.2(d)	Bond stress versus slip relationship of FRP bars with end cap anchorage	112
		4.5.2(e)	Characteristics of bond stress versus slip curves for FRP bars with end cap anchorage	114
	4.5.3	Failure m	nodes of pull-out specimens	114
4.6	Flexu	ral perforn	nance of FRP-RC and steel-RC beams	116
	4.6.1	Load def	lection diagram of RC beams with FRP bars	116
	4.6.2	Load def	lection diagram of RC beams with steel bars	119
	4.6.3	Characte different	ristics of load deflection diagram of RC beams with tbars	120
		4.6.3(a)	Ultimate load capacity	120
		4.6.3(b)	Stiffness	124
		4.6.3(c)	Mid-span deflection at ultimate load	125
		4.6.3(d)	Mid-span deflection at service load (serviceability)	126
		4.6.3(e)	Ductility	128
	4.6.4	Modes of	f failure, strains and cracking patterns	132
		4.6.4(a)	Failure mode and cracking pattern of GFRP beams	136

		4.6.4(e)	Failure mode and cracking pattern of BFRP beams	140
		4.6.4(c)	Failure mode and cracking pattern of CFRP beams	144
		4.6.4(d)	Failure mode and cracking pattern of steel-RC beams	148
4.7	Summ	nary		150
CH	АРТЕБ	R FIVE: T	HEORETICAL STUDY	
5.1	Introd	uction		151
5.2	FRP to	o concrete	bond empirical modelling	151
5.3		_	iction of shear and flexural behavior of concrete or steel bars	159
	5.3.1	Theoretic	al shear strength of FRP-RC beams	159
	5.3.2	Theoretic	al shear strength of steel-RC beams	160
	5.3.3	Theoretic	al flexural strength of FRP-RC beams according to R	161
		5.3.3(a)	Calculations steps for flexural capacity of beams with FRP bars - concrete crushing failure case ($\rho f > \rho b$)	165
		5.3.3(b)	Calculations steps for flexural capacity of beams with FRP bars – FRP rupture failure case (ρf>ρb)	165
		5.3.3(c)	Calculations steps for flexural capacity of beams with FRP bars - bond failure case	167
	5.3.4	Theoretic	al prediction of cracking load	171
	5.3.5	Theoretic section	al prediction of moment of inertia of the cracked	172
	5.3.6	Theoretic	al prediction of mid-span deflection of FRP-RC beams	173
5.4	Comp	arison betw	ween theory and experiments	176
	5.4.1	Ultimate	load capacity	176
	5.4.2	Mid-span	defection	178

5.4.3 Cracking load	180		
5.4.4 Failure modes	181		
5.4.5 Strains of concrete and reinforcement at failure	182		
CHAPTER SIX: CONCLUSIONS AND RECOMMENDATIONS			
6.1 Introduction	184		
6.2 Conclusions	184		
6.3 Recommendations	188		
REFERENCES	189		
APPENDICES			
Appendix A: Tables and Figures			
Appendix B: Theoretical prediction of flexural capacity of FRP-RC beams			
Appendix C: Theoretical prediction of flexural capacity of steel-RC beams			

LIST OF TABLES

		Page
Table 2.1	Mechanical properties of FRP composites and mild steel available in the literature as summarized by Kodur and Baingo, (1998)	11
Table 2.2	Thermal properties of FRP composites and mild steel available in the literature as summarized by Kodur and Baingo, (1998)	11
Table 2.3	Different research studies that carried out on bond behavior between FRP composites and concrete	15
Table 2.4	Different research studies that carried out on FRP-RC structures	21
Table 2.5	Different research works that carried out on the thermal properties of FRP composites under high temperatures	29
Table 2.6	Different research works that carried out on the mechanical properties of FRP composites under high temperatures	31
Table 2.7	Reduction factors $k_{\rm f}$ and $k_{\rm E}$ as proposed by Nadjai et al., (2005)	34
Table 2.8	Coefficient A, B, C, and n as proposed by Wang et al., (2011)	34
Table 2.9	Different research works that carried out on the effect of high temperatures on the bond between FRP bars and concrete	38
Table 2.10	Different research works that carried out on fire tests on FRP-RC structures	42
Table 2.11	Different research works that carried out on the behavior of concrete under high temperatures	54
Table 3.1	Mechanical properties of different used FRP bars as per manufacturer data sheet	61

Table 3.2	The mechanical properties of reinforcing steel bars collected from our experimental work	62
Table 3.3	Concrete mix proportioning, based upon Ismail et al., (2011)	62
Table 3.4	Properties of fine and coarse aggregates	64
Table 3.5	Properties of CONCRESIVE 1441S epoxy as provided by BASF-Malaysia	65
Table 3.6	Duralco 4703 epoxy properties	65
Table 3.7	Properties of SS316L stainless steel used as per supplier data sheet	65
Table 3.8	Temperature reduction for different insulation thicknesses by special coating as per the manufacturer's technical data sheet	66
Table 4.1	Average characteristics of stress-strain diagram for different FRP bars at different temperatures (mean \pm standard deviation)	98
Table 4.2	Steel bars-tensile tests results (mean ± standard deviation)	105
Table 4.3	Compressive and tensile strength of concrete at 23°C and 500°C	108
Table 4.4	Characteristics of bond stress versus strain curves for pullout specimens with different bars and exposure temperatures	113
Table 4.5	Characteristics of load-deflection diagram for RC beams with different FRP bars, without and with end caps, before and after heating to 500°C	123
Table 4.6	Observed modes of failure	133
Table 4.7	Strain readings of concrete and different reinforcing bars at ultimate loads	134
Table 5.1	Regression constants for predicting α_T and β_T (Eqs. 5.4 & 5.5)	152

Table 5.2	Experimental and analytical FRP's maximum bond stress and corresponding slip at different temperatures	154
Table 5.3	Theoretical shear capacity of RC beams at different conditions	161
Table 5.4	FRP properties at 23°C and 325°C	170
Table 5.5	Reinforcement ratio for FRP-RC beams	170
Table 5.6	Maximum bond forces for FRP-RC beams based on bond strength results	171
Table 5.7	Theoretical flexural capacities of all RC beams as per ACI codes and predicted flexural capacities based on bond strength results	171
Table 5.8	Theoretical cracking load of all RC beams, kN	172
Table 5.9	Cracked moment of inertia for different RC beams	173
Table 5.10	Theoretical mid-span deflection of FRP-RC beams at different load stages	176
Table 5.11	Theoretical mid-span deflection of steel-RC beams as per ACI-318	176
Table 5.12	Comparison between beams' theoretical and experimental load capacities	177
Table 5.13	Theoretical and experimental mid-span deflection at cracking, ultimate and service loads for different cases of RC beams	180
Table 5.14	Theoretical and experimental cracking load for different RC beams	181
Table 5.15	Theoretical, expected and observed failure modes	182
Table 5.16	Experimental and theoretical strains of concrete and reinforcement at ultimate load	183

LIST OF FIGURES

		Page
Figure 2.1	Ductility index explanation proposed by Naaman and Jeong, (1995)	20
Figure 2.3	Effect of temperature on thermal properties of GFRP proposed by Bai et al., (2007a)	30
Figure 2.3	Effect of temperature on strength of different construction materials, presented by Kodur and Baingo, (1998)	35
Figure 2.4	Effect of temperature on elastic modulus and effective coefficient of thermal expansion of GFRP, proposed by Bai et al., (2007b)	35
Figure 2.5	Effect of temperature on FRP's creep strain under constant tensile loading 76 MPa, presented by Gates, (1991)	36
Figure 2.6	Temperature vs time as a function of concrete cover, presented by Nigro et al., 2011a	51
Figure 3.1	Flow chart of methodology	60
Figure 3.2	Different reinforcement bars used in this study	62
Figure 3.3	SS316L End Cap	66
Figure 3.4	Different FRP bars with steel end caps, with and without thermal insulation coating (RLHY12)	67
Figure 3.5	Detailing of testing program via tensile test for FRP/Steel bars	68
Figure 3.6	Detailing of testing program via pullout tests	69
Figure 3.7	Detailing of testing program for different beams in flexure	69
Figure 3.8	Damaged FRP bars after exposure to 450°C	70

Figure 3.9	GFRP bars before and after exposure to different high temperatures	71
Figure 3.10	BFRP bars before and after exposure to different high temperatures	71
Figure 3.11	CFRP bars before and after exposure to different high temperatures	72
Figure 3.12	Steel bars after exposure to different elevated temperatures ready for tensile test	72
Figure 3.13	Samples of FRP bars before and after exposure to different elevated temperatures, ready for tensile strength test	73
Figure 3.14	Samples of FRP bars before and after exposure to different elevated temperatures, ready for elastic modulus determination	73
Figure 3.15	Detailing of the geometric dimensions of pullout test specimen molds	75
Figure 3.16	Detailing of pullout specimens with steel end cap anchorage	75
Figure 3.17	Wooden molds for concrete beam	76
Figure 3.18	Reinforcement details	77
Figure 3.19	Typical reinforcement cages with and without steel caps	77
Figure 3.20	Reinforcement cages inside the wooden molds	78
Figure 3.21	Strain gauge at reinforcement bar	78
Figure 3.22	Hole at bottom of RC beam used for later fixation of strain gauges	79
Figure 3.23	Fixation of strain gauge after burning of RC beam	79

Figure 3.24	Locations of thermocouples	80
Figure 3.25	Strain gauge at beam's mid-span top surface	80
Figure 3.26	Slump test	82
Figure 3.27	RC beams after casting in the wooden molds	82
Figure 3.28	Water spray curing of RC beams	82
Figure 3.29	Heat scheme for pullout specimens	84
Figure 3.30	Pullout specimen with external thermal insulation coating protection	84
Figure 3.31	Pullout specimens in gas furnace after heating	85
Figure 3.32	Pullout specimen with top steel pipe ready for testing	84
Figure 3.33	Heating time-temperature schedule	86
Figure 3.34	Two RC beam inside the furnace ready for heating	87
Figure 3.35	FRP/steel bars attached to the jaws of the testing machine using epoxy bonded pipes	88
Figure 3.36	Testing setup for obtaining the exact stress-strain diagram for Steel/FRP bars using an extensometer	89
Figure 3.37	Pullout test arrangement	90
Figure 3.38	Flexural test arrangement, setup and positioning of the LVDT's	91
Figure 3.39	Cube compressive strength test	92
Figure 3.40	Concrete cylinder compressive strength test	92
Figure 3.41	Concrete cylinder splitting tensile strength test	93

Figure 4.1	Temperature profile for RC beams heated to 500°C	96
Figure 4.2	Typical stress-strain curves of GFRP bars at different temperatures	99
Figure 4.3	Typical stress-strain curves of BFRP bars at different temperatures	99
Figure 4.4	Typical stress-strain curves of CFRP bars at different temperatures	99
Figure 4.5	Failure of heated FRP bar in tensile testing	101
Figure 4.6	Residual tensile strength for FRP bars under elevated temperatures	102
Figure 4.7	Residual elastic modulus for FRP bars under elevated temperatures	103
Figure 4.8	Typical stress-strain curves of steel bars at different temperatures	105
Figure 4.9	Normalized tensile strength of steel bars	`106
Figure 4.10	Normalized elastic modulus of steel bars	106
Figure 4.11	Compressive stress versus strain for concrete before and after exposure to 500°C	107
Figure 4.12	Typical bond-slip curves of FRP and steel bars at ambient temperature	109
Figure 4.13	Typical bond stress-slip curves of pullout specimens with GFRP bars subjected to different temperatures	110
Figure 4.14	Typical bond stress- slip curves of pullout specimens with BFRP bars subjected to different temperatures	111

Figure 4.15	Typical bond stress- slip curves of pullout specimens with CFRP bars subjected to different temperatures	111
Figure 4.16	Typical bond stress- slip curves of pullout specimens with steel bars subjected to different temperatures	111
Figure 4.17	Different failure modes of pullout tests	115
Figure 4.18	Typical load-deflection behavior of FRP-RC beams	116
Figure 4.19	Typical load-deflection response of FRP beams with and without end caps before and after exposure to 500°C	118
Figure 4.20	Typical load-deflection curve of steel-RC beams before and after exposure to 500°C	119
Figure 4.21	Typical load-deflection curves pertaining to different FRP-RC beams (with and without end steel caps) tested in flexure before and after exposure to 500°C	122
Figure 4.22	Typical curves of load versus strain in concrete/reinforcement in different RC beams, with and without end caps, tested under flexural before and after exposure to 500°C.	135
Figure 4.23	Cracking pattern of beams with GFRP bars, without end caps, tested at 23°C	138
Figure 4.24	Cracking pattern of beams with GFRP bars, with end caps, tested at 23°C	138
Figure 4.25	Cracking pattern of beams with GFRP bars, without end caps, tested after exposure to 500°C	139
Figure 4.26	Cracking pattern of beams with GFRP bars, with end caps, tested after exposure to 500°C	139

Figure 4.27	Cracking pattern of beams with BFRP bars, without end caps, tested at 23°C	142
Figure 4.28	Cracking pattern of beams with BFRP bars, with end caps, tested at 23°C	142
Figure 4.29	Cracking pattern of beams with BFRP bars, without end caps, tested after exposure to 500°C	143
Figure 4.30	Cracking pattern of beams with BFRP bars, with end caps, tested after exposure to 500°C	143
Figure 4.31	Cracking pattern of beams with BFRP bars, without end caps, tested at 23°C	146
Figure 4.32	Cracking pattern of beams with CFRP bars, with end caps, tested at 23°C	146
Figure 4.33	Cracking pattern of beams with CFRP bars, without end caps, tested after exposure to 500°C	147
Figure 4.34	Cracking pattern of beams with CFRP bars, with end caps, tested after exposure to 500°C	147
Figure 4.35	Cracking pattern of beams with steel bars, tested at 23°C	149
Figure 4.36	Cracking pattern of beams with steel bars, tested after exposure to 500°C	149
Figure 5.1	Experimental and empirical ascending portion of bond-slip curves for pullout specimens with GFRP bars exposed to different temperatures	155
Figure 5.2	Experimental and empirical ascending portion of bond-slip curves for pullout specimens with BFRP bars exposed to different temperatures	156

Figure 5.3	Experimental and empirical ascending portion of bond-slip curves for pullout specimens with CFRP bars exposed to different temperatures	157
Figure 5.4	Empirical ascending portion of bond-slip curves for pullout specimens with GFRP bars exposed to different temperatures	158
Figure 5.5	Empirical ascending portion of bond-slip curves for pullout specimens with BFRP bars exposed to different temperatures	158
Figure 5.6	Empirical ascending portion of bond-slip curves for pullout specimens with CFRP bars exposed to different temperatures	158
Figure 5.7	Schematic of tested beams loading	160
Figure 5.8	Hognestad's model for concrete stress-strain relationship	163
Figure 5.9	Eurocode's model for concrete stress-strain relationship at high temperatures	164
Figure 5.10	Stresses and internal forces of FRP-RC rectangular section-case1	165
Figure 5.11	Stresses and internal forces of FRP-RC rectangular section-case2	166
Figure 5.12	Critical Section for bonding failure	168

LIST OF ABBREVIATIONS

AFRP Aramid Fiber Reinforced Polymer

BE Bar Elongation

B.S. Bond Slip Failure

BFRP Basalt Fiber Reinforced Polymer

BPE Bertero- Popov-Eligehausen

C.C. Concrete Crushing Failure

C3S Portlandite

CFRP Carbon Fiber Reinforced Polymer

CH Calcium Hydroxide

CIMA Cement Industries of Malaysia Berhad

CMR Cosenza- Manfredi - Realfonzo

CSH Calcium Silicate Hydrate

CTE Coefficient of Thermal Expansion

DMA Dynamic Mechanical Analysis

DMTA Dynamic Mechanical Thermal Analysis

DSC Differential Scanning Calorimetry

DTG Derivative Thermo-Gravimetric Analysis

EC End Cap

EFM Expected Failure Mode

EPR Epoxy Polymer Resin

EUS Elongation at Ultimate Strength

FA Fly Ash

FRP Fiber Reinforced Polymer

GFRP Glass Fiber Reinforced Polymer

H.T. High Temperature

HFRHSC Hybrid-Fiber-Reinforced-High-Strength-Concrete

HPC High-Performance Concrete

HPSCC High-Performance, Self-Consolidating Concrete

HSC High Strength Concrete

I.S. Initial Stiffness

ID Inner Diameter

LVDT Linear Variable Differential Transducers

MK Metakaolin Concrete

N.A. Not Available/ Not Applicable

N.T. Normal Temperature

NSC Normal Strength Concrete

OD Outer Diameter

ODF Overall Ductility Factor

OPC Ordinary Portland Cement

PFA Pulverized Fly Ash

PVC Polyvinyl Chloride polymer

RC Reinforced Concrete

RUBS Residual Ultimate Bond Strength

S Shear Failure

S.R. Reduction in Stiffness

S.Y. Steel Yielding Followed by Concrete Crushing

SABS Slip at Maximum Bond Strength

SAF Stroke at Maximum Force

SEM Scanning Electronic Microscopy

SF Silica Fume

Sp. # Specimen Number

SS316L Stainless Steel Grade 316L

T.C. Thermocouple

TGA Thermo-Gravimetric Analysis

UBS Ultimate Bond Strength

UD Uni-directional

UP Unsaturated Polyester

US Ultimate Strength

VER Vinyl Ester Resign

YS Yield Strength