
UNIVERSITI SAINS MALAYSIA

Second Semester Examination
2015/2016 Academic Session

June 2016

EAS254 – Structural Analysis
[Analisis Struktur]

Duration : 3 hours
[Masa : 3 jam]

Please check that this examination paper consists of **NINE (9)** pages of printed material including **ONE (1)** appendix before you begin the examination.

*[Sila pastikan bahawa kertas peperiksaan ini mengandungi **SEMBILAN (9)** muka surat yang bercetak termasuk **SATU (1)** lampiran sebelum anda memulakan peperiksaan ini.]*

Instructions : This paper contains **SIX (6)** questions. Answer **FIVE (5)** questions.

Arahan : Kertas ini mengandungi **ENAM (6)** soalan. Jawab **LIMA (5)** soalan.]

All questions **MUST BE** answered on a new page.

*[Semua soalan **MESTILAH** dijawab pada muka surat baru.]*

In the event of any discrepancies, the English version shall be used.

[Sekiranya terdapat percanggahan pada soalan peperiksaan, versi Bahasa Inggeris hendaklah diguna pakai.]

1. [a] Determine the vertical and horizontal displacements at point C for the truss shown in **Figure 1**. The cross-sectional area of each member is given in parentheses (mm^2) and the modulus of elasticity is 200 GPa. Use virtual work method.

*Kira anjakan tegak dan ufuk di titik C untuk kekuda yang ditunjukkan dalam **Rajah 1**. Luas keratan rentas setiap ahli diberikan dalam kurungan (mm^2) dan modulus keanjalan ialah 200 GPa. Guna kaedah kerja maya.*

[16 marks/markah]

- [b] If member BC is short by 5 mm and it is forced into place, calculate the vertical and horizontal displacements at point C due to the fabrication error and the external load.

Jika anggota BC adalah pendek sebanyak 5 mm dan dipaksa dipasangkan, kira anjakan tegak dan ufuk di titik C akibat kesilapan fabrikasi dan beban luaran.

[4 marks/markah]

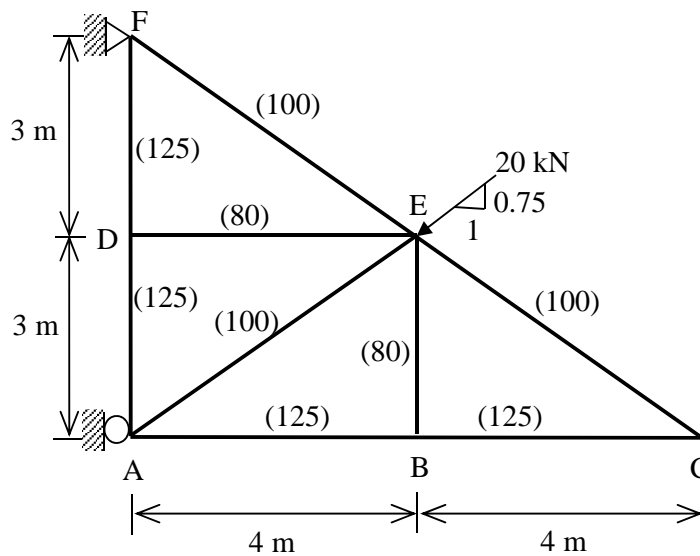


Figure 1 / Rajah 1

2. A two-span continuous steel beam as shown in **Figure 2** is supporting two uniformly distributed loads of 30 kN/m and 20 kN/m along span AB and BC, respectively. The continuous beam is supported by a pin at A and by a roller at B and C. Determine the reaction at all supports A, B and C of the continuous beam by the method of least work. The span AB and BC of the beams have the second moment of area of 79100 cm⁴ and 57100 cm⁴, respectively. The elastic modulus of the steel beam is 200 GN/m². Ignore the self-weight of the beam.

*Satu rasuk keluli selangar dua-rentang seperti ditunjukkan dalam **Rajah 2** menyokong dua beban teragih seragam sebanyak masing-masing 30 kN/m dan 20 kN/m sepanjang rentang AB dan BC. Rasuk selangar tersebut disokong oleh satu pin di A dan satu rola di B dan C. Tentukan tindakbalas di semua penyokong A, B dan C rasuk selangar tersebut menggunakan kaedah kerja terkurang. Rentang AB dan BC rasuk tersebut mempunyai momen luas kedua masing-masing 79100 cm⁴ dan 57100 cm⁴. Modulus keanjalan rasuk keluli ialah 200 GN/m². Abaikan berat-diri rasuk tersebut.*

[20 marks/markah]

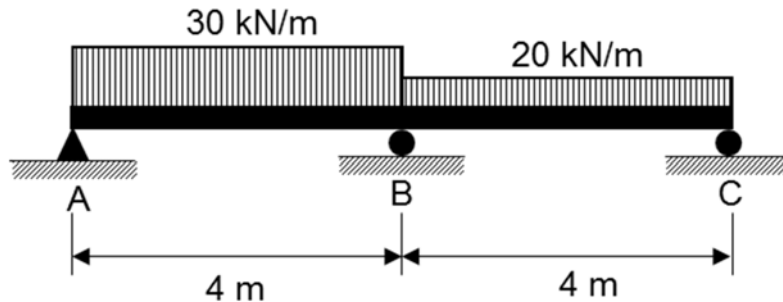


Figure 2 / Rajah 2

3. **Figure 3** shows a frame carrying a uniformly distributed load of 30 kN/m for member DBC, with overhang portion DB. Supports A and C are fixed. The flexural rigidity of member DBC and AB are 4I and I, respectively. E is constant for the frame.

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Rajah 3 menunjukkan satu kerangka dengan bahagian terjulur DB membawa daya teragih seragam sebanyak 30 kN/m bagi rentang DBC. Penyokong A dan C adalah jenis terikat. Ketegaran lenturan bagi rentang DBC dan AB adalah masing-masing $4I$ dan I . E adalah malar untuk kerangka tersebut.

[a] Compute the internal moments at the joint of the frame by using Slope Deflection Method. Fixed end moment is given in the **Appendix**.

Kira nilai momen dalaman di setiap sambungan kerangka tersebut dengan menggunakan Kaedah Cerun Pesongan. Momen terikat hujung diberikan dalam **Lampiran**.

[15 marks/markah]

[b] Draw the shear force and bending moment diagrams for member BC.

Lukiskan gambarajah daya ricih dan momen lentur bagi anggota BC.

[5 marks/markah]

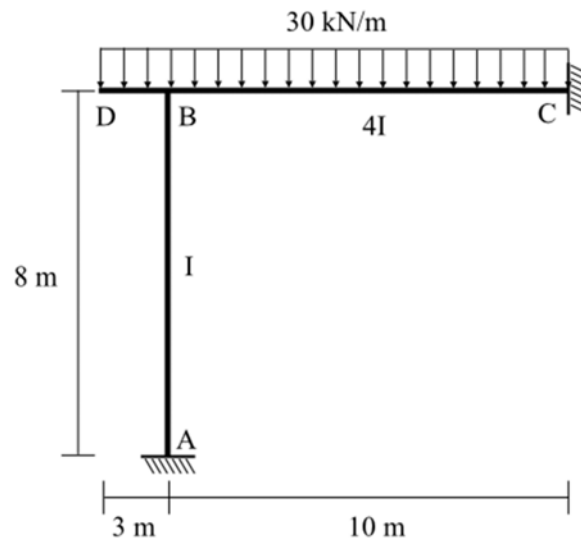


Figure 3 / Rajah 3

4. **Figure 4** shows a beam carrying a uniformly distributed load of 40 kN/m for span BC and a point load of 25 kN at the end of cantilever CD. Support A is fixed, whereas supports B and C are roller. Assume EI is constant for the beam.

Rajah 4 menunjukkan rasuk yang membawa beban teragih seragam sebanyak 40 kN/m bagi rentang BC dan beban tumpu sebanyak 25 kN di hujung rentang julur CD. Penyokong A adalah terikat tegar manakala B dan C adalah rola. Anggap EI adalah malar untuk rasuk tersebut.

- [a] Compute the internal moments at the joint of the beam by using Moment Distribution Method. Fixed end moment is given in the **Appendix**.

Kira nilai momen dalaman di setiap sambungan rasuk tersebut dengan menggunakan Kaedah Agihan Momen. Momen terikat hujung diberikan dalam Lampiran.

[16 marks/markah]

- [b] Sketch the bending moment diagram and the qualitative deflected shape for the beam.

Lakarkan gambarajah momen lentur dan bentuk pesongan kualitatif bagi rasuk tersebut.

[4 marks/markah]

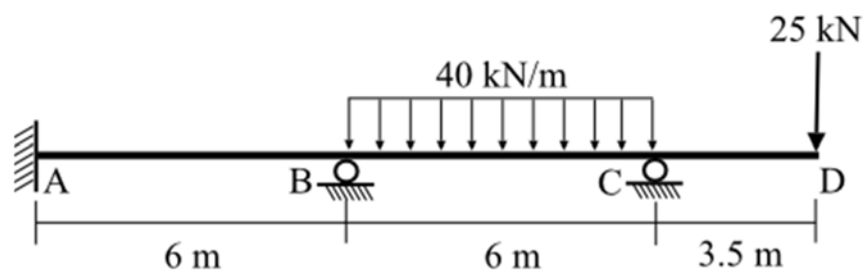


Figure 4 / Rajah 4

5. A rigid-jointed frame is designed to carry the working loads as shown in **Figure 5**. If the collapse load factor is 1.8, determine the required plastic moment capacity (M_p) for all independent mechanisms and any five combined mechanisms.

*Sebuah kerangka terikat tegar direkabentuk untuk menanggung beban kerja seperti ditunjukkan dalam **Rajah 5**. Jika faktor beban runtuh ialah 1.8, tentukan kapasiti momen plastik (M_p) yang diperlukan untuk semua mekanisma bebas dan mana-mana lima mekanisma gabungan.*

[20 marks/markah]

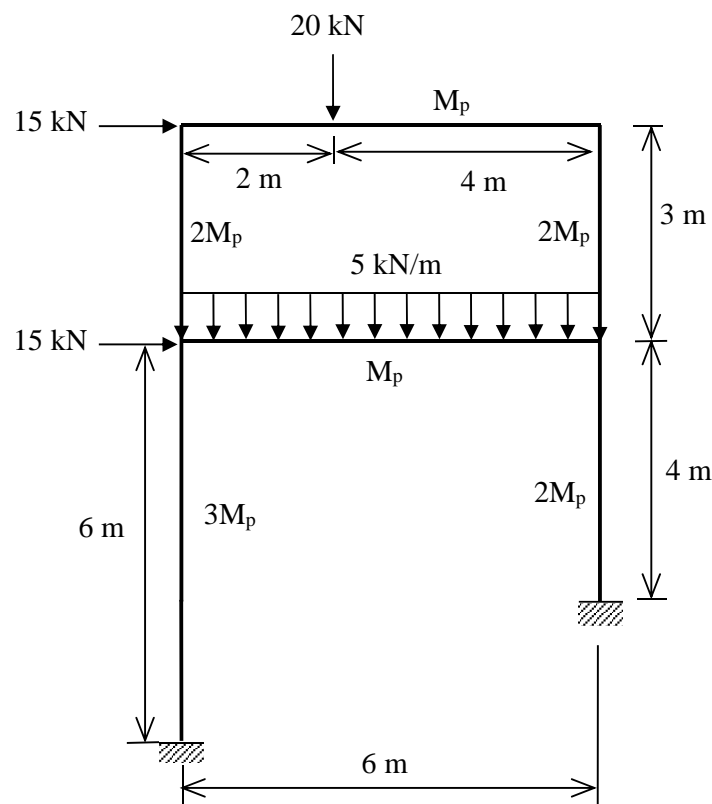


Figure 5 / Rajah 5

6. [a] **Figure 6(a)** shows a portal frame that carries uniformly distributed load of 5 kN/m along span BC and CD with pin and roller supports at A and E, respectively. The load is perpendicular to the spans. Two horizontal concentrated loads of 15 kN are acting at joints B and D. Use the virtual work method to determine the vertical deflection at joint C of the frame shown in **Figure 6(a)**. Take $E = 200 \text{ GPa}$ and $I = 600 \times 10^6 \text{ mm}^4$.

Rajah 6(a) menunjukkan kerangka portal yang menanggung beban teragih seragam sebanyak 5 kN/m di sepanjang rentang BC dan CD dengan penyokong A dan E masing-masing adalah pin dan rola. Beban tersebut adalah serenjang terhadap rentang. Dua beban tumpu ufuk 15 kN bertindak di sambungan B dan D. Guna kaedah kerja maya untuk kira anjakan tegak di sambungan C untuk kerangka yang ditunjukkan dalam *Rajah 6(a)*. Guna $E = 200 \text{ GPa}$ and $I = 600 \times 10^6 \text{ mm}^4$.

[10 marks/markah]

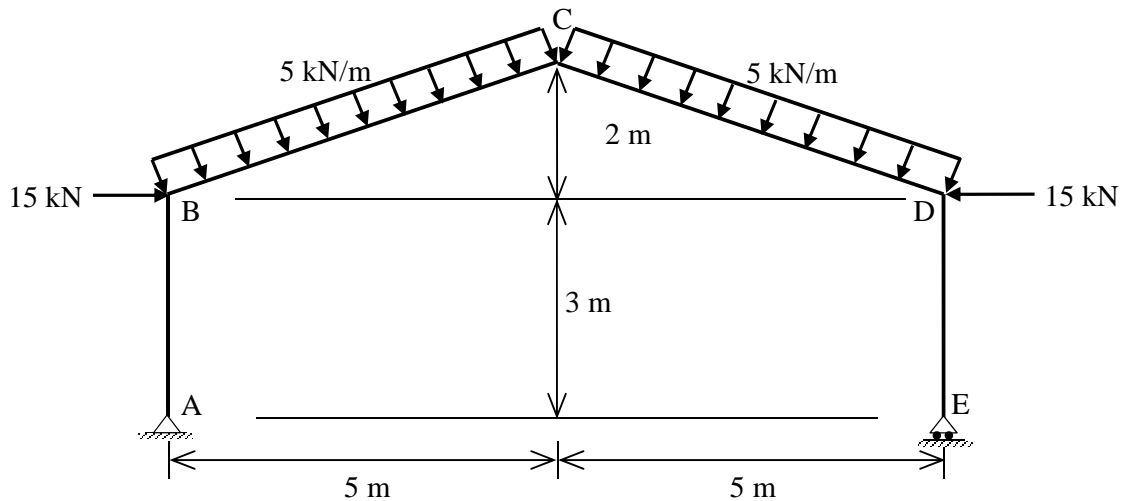


Figure 6(a) / Rajah 6(a)

- [b] The section of the steel beam for a frame is shown in **Figure 6(b)**. Determine the plastic moment capacity, elastic moment capacity and shape factor of the beam section. The yield stress of steel is 275 N/mm^2 .

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Keratan rasuk keluli untuk kerangka diberikan dalam **Rajah 6(b)**. Kira kapasiti momen plastik, kapasiti momen elastik dan faktor bentuk keratan rasuk. Tegasan alah untuk keluli ialah 275 N/mm^2 .

[10 marks/markah]

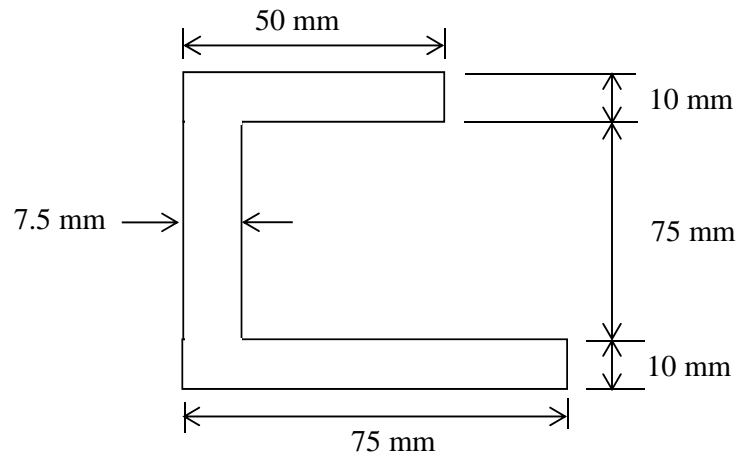
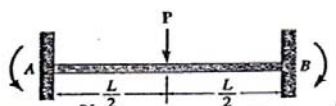
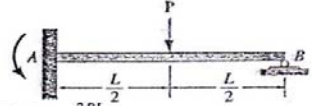
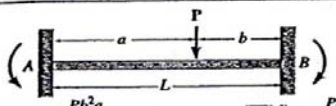
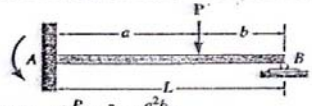
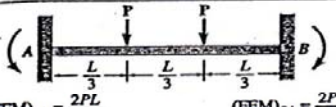
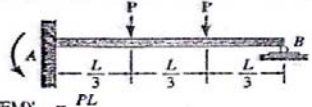
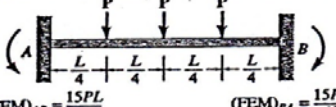
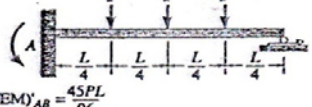
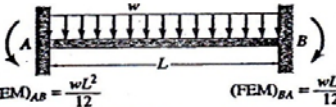
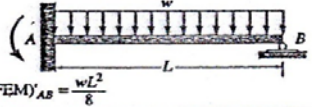
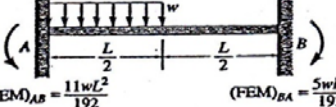
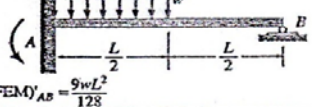
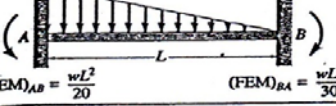
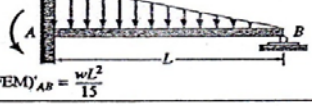
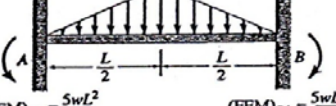
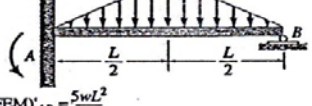




Figure 6(b) / Rajah 6(b)

APPENDIX / LAMPIRAN

Fixed End Moments

| | |
|---|--|
|  <p> $(FEM)_{AB} = \frac{PL}{8}$ $(FEM)_{BA} = \frac{PL}{8}$ </p> |  <p> $(FEM)'_{AB} = \frac{3PL}{16}$ </p> |
|  <p> $(FEM)_{AB} = \frac{Pb^2a}{L^2}$ $(FEM)_{BA} = \frac{Pa^2b}{L^2}$ </p> |  <p> $(FEM)'_{AB} = \left(\frac{P}{L^2}\right)(b^2a + \frac{a^2b}{2})$ </p> |
|  <p> $(FEM)_{AB} = \frac{2PL}{9}$ $(FEM)_{BA} = \frac{2PL}{9}$ </p> |  <p> $(FEM)'_{AB} = \frac{FL}{3}$ </p> |
|  <p> $(FEM)_{AB} = \frac{15PL}{48}$ $(FEM)_{BA} = \frac{15PL}{48}$ </p> |  <p> $(FEM)'_{AB} = \frac{45PL}{96}$ </p> |
|  <p> $(FEM)_{AB} = \frac{wL^2}{12}$ $(FEM)_{BA} = \frac{wL^2}{12}$ </p> |  <p> $(FEM)'_{AB} = \frac{wL^2}{8}$ </p> |
|  <p> $(FEM)_{AB} = \frac{11wL^2}{192}$ $(FEM)_{BA} = \frac{5wL^2}{192}$ </p> |  <p> $(FEM)'_{AB} = \frac{9wL^2}{128}$ </p> |
|  <p> $(FEM)_{AB} = \frac{wL^2}{20}$ $(FEM)_{BA} = \frac{wL^2}{30}$ </p> |  <p> $(FEM)'_{AB} = \frac{wL^2}{15}$ </p> |
|  <p> $(FEM)_{AB} = \frac{5wL^2}{96}$ $(FEM)_{BA} = \frac{5wL^2}{96}$ </p> |  <p> $(FEM)'_{AB} = \frac{5wL^2}{64}$ </p> |
|  <p> $(FEM)_{AB} = \frac{6EI\Delta}{L^2}$ $(FEM)_{BA} = \frac{6EI\Delta}{L^2}$ </p> |  <p> $(FEM)'_{AB} = \frac{3EI\Delta}{L^2}$ </p> |