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## UNIVERSITI SAINS MALAYSIA

First Semester Examination  
2015/2016 Academic Session

December 2015 / January 2016

### **EAS353 – Reinforced Concrete Structural Design 1** *[Rekabentuk Struktur Konkrit Bertetulang 1]*

Duration : 2 hours  
*[Masa : 2 jam]*

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Please check that this examination paper consists of **EIGHT (8)** pages of printed material including **ONE (1)** appendix before you begin the examination.

*[Sila pastikan bahawa kertas peperiksaan ini mengandungi **LAPAN (8)** muka surat yang bercetak termasuk **SATU (1)** lampiran sebelum anda memulakan peperiksaan ini.]*

**Instructions:** This paper contains **FIVE (5)** questions. Answer **FOUR** questions.

**[Arahan :** Kertas ini mengandungi **LIMA (5)** soalan. Jawab **EMPAT** soalan.

You may answer the question either in Bahasa Malaysia or English.

*[Anda dibenarkan menjawab soalan sama ada dalam Bahasa Malaysia atau Bahasa Inggeris].*

All questions **MUST BE** answered on a new page.

*[Semua soalan **MESTILAH** dijawab pada muka surat baru].*

In the event of any discrepancies, the English version shall be used.

*[Sekiranya terdapat percanggahan pada soalan peperiksaan, versi Bahasa Inggeris hendaklah diguna pakai].*

1. [a] A 3-span continuous beam of 250 mm x 650 mm, and each span of 5.0 m as shown in **Figure 1**. The slab thickness is 180 mm. In the transverse direction, the beams spacing (B) are at 4.0 m centres with the supports of 250 mm.

The permanent load is 30 kN/m including self weight and the variable load is 12.5 kN/m. The characteristic strength of concrete is  $f_{ck} = 30 \text{ N/mm}^2$  and characteristic strength of steel is  $f_{yk} = 500 \text{ N/mm}^2$ .

Assume that the nominal cover is 30 mm, nominal aggregate size is 20 mm and the fire resistance is for 1 hour. Life of building is 50 years. Use 16 mm and 8 mm size of longitudinal and shear reinforcement, respectively.

Design the reinforcements for bending and shear for the beam.

*Sebuah rasuk 3-rentang selanjur berdimensi 250 mm x 650 mm, dan setiap rentang 5.0 m seperti di **Rajah 1**. Ketebalan papak ialah 180 mm. Di arah melintang, jarak rasuk (B) ialah 4.0 m pusat ke pusat. Manakala lebar penyokong adalah 250 mm.*

*Beban tetap adalah 30 kN/m termasuk beban diri dan beban boleh ubah ialah 12.5 kN/m. Ciri kekuatan konkrit  $f_{ck} = 30 \text{ N/mm}^2$  dan ciri kekuatan keluli  $f_{yk} = 500 \text{ N/mm}^2$ .*

*Andaikan penutup konkrit nominal ialah 30 mm, saiz nominal agregat 20 mm dan rintangan kebakaran ialah 1 jam. Hayat rekabentuk adalah 50 tahun. Guna saiz 16 mm dan 8 mm masing-masing untuk tetulang membujur dan tetulang ricih.*

*Rekabentuk tetulang lenturan dan ricih untuk rasuk.*

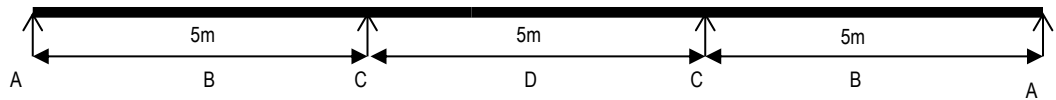


Figure 1/Rajah 1

[25 marks/markah]

2. [a] A free body diagram of continuous beam as shown in **Figure 2** has a constant cross-section and supports uniformly distributed permanent action of 30 kN/m and a variable action of 10 kN/m. Establish four critical loading patterns in accordance with EC2 for analysis of structure.

*Sebuah gambarajah jasad bebas rasuk selanjur ditunjukkan dalam **Rajah 2** mempunyai keratan rentas sekata dan menanggung beban tetap teragih seragam 30 kN/m dan beban boleh ubah 10 kN/m. Tentukan empat susunan beban kritikal berdasarkan EC2 untuk analisis struktur.*

[8 marks/markah]

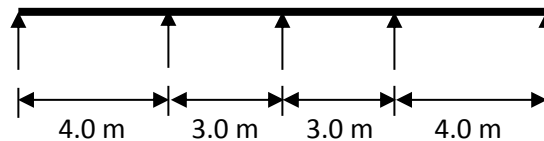


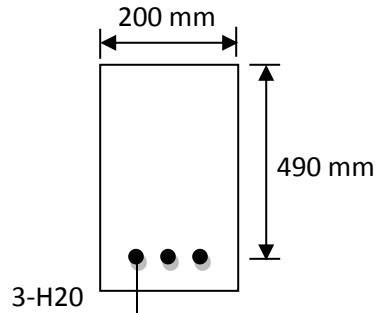
Figure 2/Rajah 2

- [b] Determine the ultimate moment of resistance of the cross section shown in **Figure 3**. Given, the characteristic strengths of the reinforcement and concrete are 500 N/mm<sup>2</sup> and 30 N/mm<sup>2</sup>, respectively. The tension reinforcement provided is 3-H20.

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Tentukan rintangannya momen maksimum keratan rentas yang ditunjukkan dalam **Rajah 3**. Diberikan, kekuatan ciri tetulang dan konkrit masing-masing ialah  $500 \text{ N/mm}^2$  dan  $30 \text{ N/mm}^2$ . Tetulang tegangan yang disediakan ialah 3-H20.

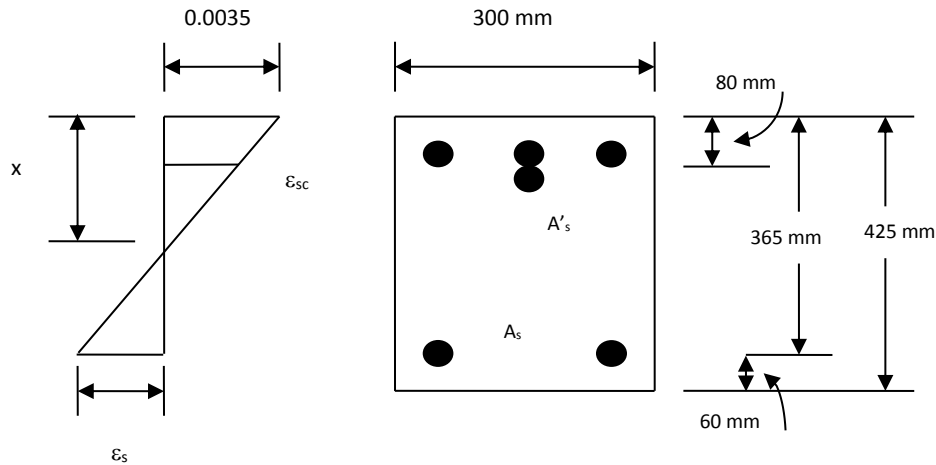
[17 marks/markah]

**Figure 3/Rajah 3**

3. A rectangular column section as shown in **Figure 4** is subjected to an axial load of 1200 kN and a moment of 240 kNm at the Ultimate Limit State (ULS). The arrangement of the reinforcement is non-symmetrical which lateral forces acting in the direction. Assuming the characteristic material strengths are  $f_{yk} = 500 \text{ N/mm}^2$  and  $f_{ck} = 30 \text{ N/mm}^2$ , determine if the areas of reinforcement required are sufficient to carry the above mentioned loads.

*Keratan tiang segiempat tepat seperti yang ditunjukkan dalam **Rajah 4** dikenakan beban paksi sebesar 1200 kN dan momen 240 kNm pada keadaan Had Muktamad. Susunatur tetulang adalah tak simetri iaitu daya-daya sisi bertindak dalam arah. Anggapkan kekuatan ciri bahan  $f_{yk} = 500 \text{ N/mm}^2$  dan  $f_{ck} = 30 \text{ N/mm}^2$ , tentukan sama ada luas tetulang yang diperlukan mencukupi untuk membawa beban-beban dinyatakan di atas.*

[25 marks/markah]



**Figure 4/Rajah 4:** Cross section of 300 x 425 mm rectangular column/  
*Keratan rentas 300 x 425 mm tiang segiempat*

4. [a] Foundation is an importance structure which transmits the designed loads from the structure to the soil or rock. Sketch **FIVE (5)** types of reinforced concrete foundation for building.

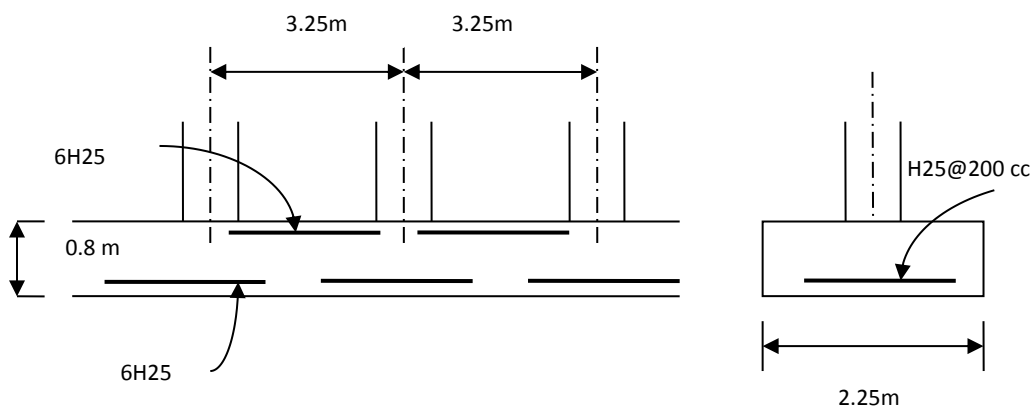
*Asas adalah struktur yang penting untuk menyalurkan beban rekabentuk daripada struktur ke tanah atau batu. Lakarkan **LIMA (5)** jenis asas konkrit bertetulang untuk bangunan.*

[5 marks/markah]

- [b] A strip footing is required to carry 350 mm square columns which are equally spaced at 3.25 m centres (see **Figure 5**). Each column carries a design load of 1800 kN. The safe bearing pressure of soil is 200 kN/m<sup>2</sup> and the characteristic material strengths are  $f_{ck} = 30 \text{ N/mm}^2$  and  $f_{yk} = 500 \text{ N/mm}^2$ . Based on the geometry and reinforcements shown in **Figure 5**, justify that the punching shear resistance, longitudinal and transverse reinforcements are designed adequately.

Asas jalur diperlukan untuk membawa tiang segiempat sama 350 mm pada jarak ruang yang sama 3.25 m pusat ke pusat (lihat **Rajah 5**). Setiap tiang membawa beban rekabentuk sebesar 1800 kN. Tekanan galas selamat tanah ialah 200 kN/m<sup>2</sup> dan kekuatan ciri bahan ialah  $f_{ck} = 30 \text{ N/mm}^2$  and  $f_{yk} = 500 \text{ N/mm}^2$ . Berdasarkan geometri dan tetulang yang ditunjukkan dalam **Rajah 5**, wajarkan bahawa rintangan ricih tebukan, tetulang membujur dan melintang yang direkabentuk adalah mencukupi.

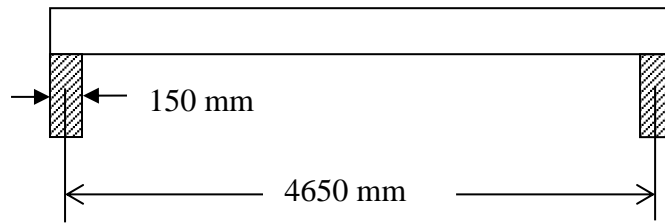
[20 marks/markah]



**Figure 5/Rajah 5:** Strip footing with longitudinal and transverse reinforcements/ Asas jalur dengan tetulang membujur dan melintang

5. Design the unrestrained simply supported floor shown in **Figure 6** when subjected to variable load of 4 kN/m<sup>2</sup>. Given the characteristic strength of concrete,  $f_{ck} = 25 \text{ N/mm}^2$ , characteristic strength of main reinforcement,  $f_{yk} = 500 \text{ N/mm}^2$ , and nominal cover to reinforcement,  $c = 25 \text{ mm}$ .

Rekabentuk lantai tidak dikekang disokong mudah seperti yang di tunjukkan dalam **Rajah 6** yang dikenakan beban boleh ubah 4 kN/m<sup>2</sup>. Diberikan kekuatan ciri konkrit,  $f_{ck} = 25 \text{ N/mm}^2$ , kekuatan ciri tetulang utama,  $f_{yk} = 500 \text{ N/mm}^2$  dan penutup nominal kepada tetulang,  $c = 25 \text{ mm}$ .



**Figure 6/Rajah 6**

[25 marks/markah]

## APPENDIX/ LAMPIRAN

**Bar Areas and Perimeters**

Sectional areas of groups of bars ( $\text{mm}^2$ )

| Bar size (mm) | Number of bars |      |      |      |      |      |      |       |       |       |
|---------------|----------------|------|------|------|------|------|------|-------|-------|-------|
|               | 1              | 2    | 3    | 4    | 5    | 6    | 7    | 8     | 9     | 10    |
| 6             | 28.3           | 56.6 | 84.9 | 113  | 142  | 170  | 198  | 226   | 255   | 283   |
| 8             | 50.3           | 101  | 151  | 201  | 252  | 302  | 352  | 402   | 453   | 503   |
| 10            | 78.5           | 157  | 236  | 314  | 393  | 471  | 550  | 628   | 707   | 785   |
| 12            | 113            | 226  | 339  | 452  | 566  | 679  | 792  | 905   | 1020  | 1130  |
| 16            | 201            | 402  | 603  | 804  | 1010 | 1210 | 1410 | 1610  | 1810  | 2010  |
| 20            | 314            | 628  | 943  | 1260 | 1570 | 1890 | 2200 | 2510  | 2830  | 3140  |
| 25            | 491            | 982  | 1470 | 1960 | 2450 | 2950 | 3440 | 3930  | 4420  | 4910  |
| 32            | 804            | 1610 | 2410 | 3220 | 4020 | 4830 | 5630 | 6430  | 7240  | 8040  |
| 40            | 1260           | 2510 | 3770 | 5030 | 6280 | 7540 | 8800 | 10100 | 11300 | 12600 |

Capacity of concrete struts expressed as a stress,  $V_{Rd,max}$

| $f_{ck}$ | $\cot \theta$ | $V_{Rd,max}$ (MPa) |            |            |            |            |            |
|----------|---------------|--------------------|------------|------------|------------|------------|------------|
|          |               | $21.8^\circ$       | $25^\circ$ | $30^\circ$ | $35^\circ$ | $40^\circ$ | $45^\circ$ |
| 25       |               | 3.10               | 3.45       | 3.90       | 4.23       | 4.43       | 4.50       |
| 30       |               | 3.64               | 4.04       | 4.57       | 4.96       | 5.20       | 5.28       |
| 35       |               | 4.15               | 4.61       | 5.21       | 5.66       | 5.93       | 6.02       |
| 40       |               | 4.63               | 5.15       | 5.82       | 6.31       | 6.62       | 6.72       |
| 45       |               | 5.09               | 5.65       | 6.39       | 6.93       | 7.27       | 7.38       |
| 50       |               | 5.52               | 6.13       | 6.93       | 7.52       | 7.88       | 8.00       |

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