$1^{\text {st }}$. Semester Examination 2000/2001 Academic Session

SEPTEMBER / OCTOBER 2000

## EAL231/3 - Transportation \& Traffic Engineering

Time : [ 3 hours ]

## Instruction to candidates:-

1. This paper consists of SEVEN (7) questions. Answer FIVE (5) questions only.
2. Answers MUST BE written in Bahasa Malaysia.
3. Your are given the following road network and information.

Kelana Jaya MRR2


Table 1

| O - D | Operated speed <br> $(\mathrm{km} / \mathrm{h})$ | $\mathrm{V} / \mathrm{c}^{*}$ | Toll <br> $(\mathrm{RM})$ | Road type |
| :--- | :---: | :---: | :---: | :--- |
| Serdang - K. Jaya | $90 \mathrm{~km} / \mathrm{h}$ | 0.48 | 2.20 | Expressway |
| Serdang - K.L. | $35 \mathrm{~km} / \mathrm{h}$ | 2.43 | - | Dual carriageway |
| Serdang - Ampang | $80 \mathrm{~km} / \mathrm{h}$ | 0.75 | - | Dual carriageway |
| K.Jaya - Sg. Buloh | $65 \mathrm{~km} / \mathrm{h}$ | 0.54 | 1.50 | Dual carriageway |
| K.L. - Sg. Buloh | $55 \mathrm{~km} / \mathrm{h}$ | 0.89 | - | Dual carriageway |
| Ampang - Sg. Buloh | $75 \mathrm{~km} / \mathrm{h}$ | 0.75 | - | Expressway |

*Note: $\mathrm{V} /{ }_{\mathrm{c}}=$ Volume per capacity ratio
(a) Serdang - K.L. - Sg. Buloh has been the old route to go from south to the north. With the MRR2 built and operational, travellers have new alternatives to get from Serdang to Sg. Buloh. Find the savings in generalised cost of travel for using the alternative routes and select the route with the lowest generalised cost of travel.
(10 marks)
(b) State the assumptions used to answer question (a).
( 5 marks)
(c) Explain the likely change in the $\mathrm{v} / \mathrm{c}$ values when the public begin to use the alternative routes.
2. You are required to construct a transportation model to represent the operational activities of buses, Light Rail Transit (LRT) and cars. To determine the generalised cost of travel for all three modes, you need to give values to the cost components.
(a) Indicate all of the required information.
(b) To acquire all the needed data requires time and cost. As a cost-effective engineer, and as a measure of cost savings, analyse all the required inputs and investigate whether the primary data acquisition can be substituted with the secondary data type and using appropriate formulae. List all the identified input and show how you intend to determine its value to be used in your model.
3. (a) One stretch of two lane two way roadway that traverses through level terrain is expected to carry 1340 vehicle per hour. What is the level of service for the road if the characteristics of the roadway are as follows:

Lane width $=11 \mathrm{ft}$
Shoulder width $=5 \mathrm{ft}$
Percent no passing zone $=10 \%$
Peak traffic volume in one direction $=1250 \mathrm{veh} / \mathrm{hr}$
Where, during the peak traffic volume, in one direction number of trucks $=540$ and number of bus $=210$. (Refer to Table 7 to Table 12 in the Appendix).
(14 marks)
(b) Observations were made at two stations XX dan YY which is located 160 m apart on a stretch of roadway. Travelling time for four vehicles traversing the two stations are shown in Table 2. If the total duration of traffic observations at station XX is 17 sec , calculate:
(i) time mean speed
(ii) space mean speed
(iii) traffic flow at station XX

Table 2 : Arrival Time (A.M.)

| Vehicle | Station XX | Station YY |
| :---: | :---: | :---: |
| A | $8: 02: 15$ | $8: 02: 22.58$ |
| B | $8: 03: 14$ | $8: 03: 23.18$ |
| C | $8: 01: 18$ | $8: 01: 25.36$ |
| D | $8: 10: 25$ | $8: 10: 34.74$ |

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8: 00: 00=8 \mathrm{Hr} 00 \mathrm{~min} 00 \mathrm{sec}
$$

4. Floating vehicle method was used to study traffic characteristics along a highway between A and B. The distance between A and B is 7.53 km . Results of the study is as shown in Table 3 and Table 4.

Table 3 : Vehicles travelling from Station A to Station B

| Start | Finish | Overtaking <br> Vehicles | Overtaken <br> Vehicles | Number of Vehicles in <br> the opposite direction |
| :---: | :---: | :---: | :---: | :---: |
| 7.02 | 7.15 | 2 | 4 | 300 |
| 7.28 | 7.40 | 3 | 3 | 360 |
| 7.50 | 8.07 | 4 | 2 | 345 |
| 8.26 | 8.40 | 5 | 2 | 410 |
| 8.50 | 9.00 | 6 | 3 | 300 |
| 9.20 | 9.35 | 1 | 2 | 340 |

Table 4 : Vehicles travelling from Station B to Station A

| Start | Finish | Overtaking <br> Vehicles | Overtaken <br> Vehicles | Number of Vehicles in <br> the opposite direction |
| :---: | :---: | :---: | :---: | :---: |
| 7.18 | 7.25 | 4 | 4 | 320 |
| 7.42 | 7.48 | 3 | 3 | 350 |
| 8.10 | 8.24 | 5 | 5 | 340 |
| 8.42 | 8.46 | 2 | 1 | 330 |
| 9.02 | 9.18 | 2 | 2 | 390 |
| 9.37 | 9.43 | 3 | 2 | 365 |

(a) Calculate the average flow and travelling time for both directions.
( 6 marks)
(b) Calculate the free flow speed and jam density for the direction from A to B.
(14 marks)
5. (a) Construction works on a roadway section causes bottleneck (closure of one lane) to the 6-lane highway (three lanes in each direction). The capacity of the road is 2450 vehicles per hour per lane. Distance between two vehicles when the traffic flow is almost stopped is 6 m . When traffic flow reaches 10,430 vehicles per hour:
(i) Calculate vehicle speed in the area far away from the bottleneck.
(ii) Calculate vehicle speed near the bottleneck.
5. (b) A Traffic survey was conducted during peak hour at one section of a roadway. Results of the survey for every 5 minutes are shown in Table 5.

Table 5

| Time | Flow (Veh/Hr) |
| :---: | :---: |
| $7.30-7.34 .9$ | 1105 |
| $7.35-7.39 .9$ | 1145 |
| $7.40-7.44 .9$ | 1040 |
| $7.45-7.49 .9$ | 1230 |
| $7.50-7.54 .9$ | 1320 |
| $7.55-7.59 .9$ | 1603 |
| $8.00-8.04 .9$ | 1430 |
| $8.05-8.09 .9$ | 1540 |
| $8.10-8.14 .9$ | 1510 |
| $8.15-8.19 .9$ | 1220 |
| $8.20-8.24 .9$ | 1033 |
| $8.25-8.29 .9$ | 980 |
| $8.30-8.34 .9$ | 990 |

(i) Sketch a histogram showing the variation in traffic flow with time.
(ii) Calculate maximum flow based on a range of 15 minutes traffic flow.
(iii) Calculate average hourly traffic and when will the peak hour occur?
(iv) Estimate the peak hour factor.
6. The geometric layout of a traffic light junction is shown in Figure 1.


Figure 1: Geometric layout of traffic light junction
Traffic volume for the junction is shown in Table 6.
a) Calculate the saturation flow for through movement and for right turning movement for the West approach.
b) Calculate saturation flow for through and right turning movements for the North approach.
c) Calculate the value of $y$ for through and right turning movement for the West approach.

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Table 6:Traffic Volume (veh/hr)

| Approach | Vehicle Type | Right Turn | Through | Left Turn |
| :--- | :--- | :--- | :--- | :--- |
| Timur | Car | 130 | 450 | 50 |
|  | Medium Lorry | 7 | 20 | 8 |
|  | Heavy Vehicle | 5 | 20 | 5 |
|  | Bus | 0 | 5 | 2 |
|  | Motorcycle | 15 | 100 | 15 |
| Belatan | Car | 120 | 510 | 45 |
|  | Medium Lorry | 6 | 70 | 12 |
|  | Heavy Vehicle | 4 | 30 | 3 |
|  | Bus | 0 | 10 | 1 |
|  | Motorcycle | 20 | 95 | 20 |
| Utara | Car | 430 | 220 | 45 |
|  | Medium Lorry | 60 | 70 | 15 |
|  | Heavy Vehicle | 25 | 30 | 0 |
|  | Bus | 10 | 5 | 1 |
|  | Motorcycle | 100 | 80 | 15 |
|  | Car | 250 | 125 | 25 |
|  | Medium Lorry | 50 | 55 | 12 |
|  | Heavy Vehicle | 20 | 25 | 1 |
|  | Bus | 2 | 7 | 0 |
|  | Motorcycle | 80 | 75 | 12 |

7. (a) Origin-Destination of roundabout junction is shown in Figure 2.
i) Calculate circulating flow and meaning flow.
ii) Based on additional data given below, calculate reserve capacity for the junction..

Diameter $=50 \mathrm{~m}$.
$\mathrm{e}=12.8 \mathrm{~m}$
$\mathrm{v}=12.5 \mathrm{~m}$
$l^{\prime}=13 \mathrm{~m}$
entry radius $=33 \mathrm{~m}$
entry angle $=32^{\circ}$
$\mathrm{K}=1-0.00347(\varnothing-30)-0.978[(1 / \mathrm{r})-0.05]$
$\mathrm{F}=303 \mathrm{X}_{2}$
$\mathrm{f}_{\mathrm{c}}=0.21 \mathrm{t}_{\mathrm{D}}\left(1+0.2 \mathrm{X}_{2}\right)$
$\left.t_{D}=1+0.5 /(1+\exp (D-60) / 10)\right)$

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\begin{aligned}
& X_{2}=v+(e-v) /(1+2 S) \\
& S=1.6(e-v) / l^{\prime}
\end{aligned}
$$



Figure 2 : Turning Volume (Veh/Hr)
(b) Discuss traffic engineering aspects that can reduce road accidents.

## APPENDIX

Table 7: Correction factor for directional distribution for two lane two way for level terrain.

| Directional <br> Split | Total Capacity (pc/hr) | Ratio of Capacity to Ideal <br> Capacity $\left(\mathrm{f}_{\mathrm{d}}\right)$ |
| :---: | :---: | :---: |
| $50 / 50$ | 2800 | 1.00 |
| $60 / 40$ | 2650 | 0.94 |
| $70 / 30$ | 2500 | 0.89 |
| $80 / 20$ | 2300 | 0.83 |
| $90 / 10$ | 2100 | 0.75 |
| $100 / 0$ | 2000 | 0.71 |

Table 8: Level of service for two lane two way road (volume vs. capacity ratio)

|  | $\begin{gathered} \hline \text { Percent } \\ \text { Time } \\ \text { Delay } \\ \hline \end{gathered}$ | Level Terrain |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Avg. Speed | Percent No Passing Zones |  |  |  |  |  |
| LOS |  |  | 0 | 20 | 40 | 60 | 80 | 100 |
| A | $\leq 30$ | $\geq 58$ | 0.15 | 0.12 | 0.09 | 0.07 | 0.05 | 0.04 |
| B | $\leq 45$ | $\geq 55$ | 0.27 | 0.24 | 0.21 | 0.19 | 0.17 | 0.16 |
| C | $\leq 60$ | $\geq 52$ | 0.43 | 0.39 | 0.36 | 0.34 | 0.33 | 0.32 |
| D | $\leq 75$ | $\geq 50$ | 0.64 | 0.62 | 0.60 | 0.59 | 0.58 | 0.57 |
| E | $>75$ | $\geq 45$ | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| F | 100 | < 45 | - | - | - | - | - | - |

Table 9 : PHF for two-lane two way road

| Total 2-Way <br> Hourly Volume <br> (vph) | PHF | Total 2-Way Hourly <br> Volume (vph) | PHF |
| :--- | :--- | :--- | :--- |
| 100 | 0.83 | 1000 | 0.93 |
| 200 | 0.87 | 1100 | 0.94 |
| 300 | 0.90 | 1200 | 0.94 |
| 400 | 0.91 | 1300 | 0.94 |


| 500 | 0.91 | 1400 | 0.94 |
| :--- | :--- | :--- | :--- |
| 600 | 0.92 | 1500 | 0.95 |
| 700 | 0.92 | 1600 | 0.95 |
| 800 | 0.93 | 1700 | 0.95 |
| 900 | 0.93 | 1800 | 0.95 |
|  |  | $\geq 1900$ | 0.96 |

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Table 10 : Correction factor for lane and shoulder width

| Usable <br> shoulder <br> Width (ft) | 12-ft Lanes |  | 11-ft lanes |  | 10-ft lanes |  | 9-ft lanes |  |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | A-D | E | LOS | LOS | LOS | LOS | LOS | LOS |
| $\geq 6$ | 1.00 | 1.00 | 0.93 | E | A-D | E | A-D | E |
| 4 | 0.92 | 0.97 | 0.85 | 0.92 | 0.84 | 0.87 | 0.70 | 0.76 |
| 2 | 0.81 | 0.93 | 0.75 | 0.88 | 0.68 | 0.85 | 0.65 | 0.74 |
| 0 | 0.70 | 0.88 | 0.65 | 0.82 | 0.58 | 0.75 | 0.57 | 0.79 |
|  |  |  | 0.66 |  |  |  |  |  |

Table 11: Average equivalent factors for trucks, recrearional vehicles and bus for two-lane two way roads.

|  |  | Type of terrain |  |  |
| :--- | :---: | :---: | :---: | :---: |
| Vehicle type | Level of <br> Service | Level | Rolling | Mountainous |
| Trucks, $\mathrm{E}_{\mathrm{T}}$ | A | 2.0 | 4.0 | 7.0 |
|  | B and C | 2.2 | 5.0 | 10.0 |
|  | D and E | 2.0 | 5.0 | 12.0 |
| RVs, $\mathrm{E}_{\mathrm{R}}$ | A | 2.2 | 3.2 | 5.0 |
|  | B and C | 2.5 | 3.9 | 5.2 |
|  | D and E | 1.6 | 3.3 | 5.2 |
| Buses, $\mathrm{E}_{\mathrm{B}}$ | A | 1.8 | 3.0 | 5.7 |
|  | B and C | 2.0 | 3.4 | 6.0 |
|  | D and E | 1.6 | 2.9 | 6.5 |

Table 12 : $\mathrm{V} / \mathrm{c}$ ratio for 2 lane -2 way road on level terrain

|  | \% no overtaking zone |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| LOS | 0 | 20 | 40 | 60 | 80 | 100 |  |
| A | 0.15 | 0.12 | 0.09 | 0.07 | 0.05 | 0.04 |  |
| B | 0.27 | 0.24 | 0.21 | 0.19 | 0.17 | 0.16 |  |


| C | 0.43 | 0.39 | 0.36 | 0.34 | 0.33 | 0.32 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| D | 0.64 | 0.62 | 0.60 | 0.59 | 0.58 | 0.57 |
| E | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| F | - | - | - | - | - | - |

## APPENDIX

Table 13

| $\mathrm{W}(\mathrm{m})$ | $\mathrm{S}(\mathrm{pcu} / \mathrm{hr})$ |
| :---: | :---: |
| 3.0 | 1845 |
| 3.25 | 1860 |
| 3.5 | 1885 |
| 3.75 | 1915 |
| 4.0 | 1965 |
| 4.25 | 2075 |
| 4.5 | 2210 |
| 4.75 | 2375 |
| 5.0 | 2560 |
| 5.25 | 2760 |

Table 14

| Gradient | Correction Factor |
| :---: | :---: |
| $+5 \%$ | 0.85 |
| $+4 \%$ | 0.88 |
| $+3 \%$ | 0.91 |
| $+2 \%$ | 0.94 |
| $+1 \%$ | 0.97 |
| $0 \%$ | 1.00 |
| $-1 \%$ | 1.03 |
| $-2 \%$ | 1.06 |
| $-3 \%$ | 1.09 |
| $-4 \%$ | 1.12 |
| $-5 \%$ | 1.15 |

Table 15

| Radius | Correction Factor |
| :--- | :--- |


| $\mathrm{R}<10 \mathrm{~m}$ | 0.85 |
| :---: | :---: |
| $10 \mathrm{~m}<\mathrm{R}<15 \mathrm{~m}$ | 0.90 |
| $15 \mathrm{~m}<\mathrm{R}<30 \mathrm{~m}$ | 0.96 |

## APPENDIX

Table 16

| \% turning <br> volume | Right turning <br> correction factor | Left turning <br> correction |
| :---: | :---: | :---: |
| 5 | 0.96 | 1.00 |
| 10 | 0.93 | 1.00 |
| 15 | 0.90 | 0.99 |
| 20 | 0.87 | 0.98 |
| 25 | 0.84 | 0.97 |
| 30 | 0.82 | 0.95 |
| 35 | 0.79 | 0.94 |
| 40 | 0.77 | 0.93 |
| 45 | 0.75 | 0.92 |
| 50 | 0.78 | 0.91 |
| 55 | 0.71 | 0.90 |
| 60 | 0.69 | 0.89 |

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