

UNIVERSITI SAINS MALAYSIA  
KAMPUS CAWANGAN PERAK

PEPERIKSAAN SEMESTER PERTAMA  
SIDANG AKADEMIK 1997/98

SEPTEMBER 1997

**EAH 322/3 - HIDROLOGI KEJURUTERAAN**

Masa : [3 jam]

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**Arahan Kepada Calon:-**

1. Sila pastikan kertas peperiksaan ini mengandungi **SEMBILAN (9)** muka surat bercetak termasuk lampiran sebelum anda memulakan peperiksaan ini.
2. Kertas ini mengandungi **TUJUH (7)** soalan. Jawab **LIMA (5)** soalan sahaja. Markah hanya akan dikira bagi **LIMA (5)** jawapan **PERTAMA** yang dimasukkan di dalam buku mengikut susunan dan bukannya **LIMA (5)** jawapan terbaik.
3. Semua soalan mempunyai markah yang sama.
4. Semua jawapan **MESTILAH** dimulakan pada muka surat yang baru.
5. Semua soalan **MESTILAH** dijawab dalam Bahasa Malaysia.
6. Tuliskan nombor soalan yang dijawab di luar kulit buku jawapan anda.

1. (a) Takrifkan istilah berikut:

- (i) Akuifer bebas;
- (ii) Akuifer terkurung; dan
- (iii) Kekonduksian hidraulik.

( 6 markah)

(b) Kirakan kekonduksian hidraulik (K) dan Keterusan (T) untuk akuifer bebas setebal 12 m jika kadar alir dari telaga ialah 360 m<sup>3</sup>/hari. Telaga cerapan 1 terletak pada jarak 20 m dari telaga pam dan surutan direkod sedalam 6 m. Manakala telaga cerapan 2 terletak pada jarak 600 m dan surutan ialah 3 m. Kedalaman airbumi yang asal ialah 12 m.

( 6 markah)

(c) Data dalam Jadual 1 telah dicatat semasa ujian pengepaman untuk telaga pengairan yang baru. Paras air yang asal ialah 5.87 m dari bahagian atas selongsong telaga. Kadar pengepaman ialah 2.0 Ml/hari. Dengan menggunakan Kaedah Jacob semi-logarithmic anggarkan:

Keterusan (T) dalam unit m<sup>2</sup>/hari dan Kebolehsimpanan (S).

| Masa dari ujian pengepaman bermula (jam) | Surutan air (meter) dari bahagian atas selongsong telaga |
|--|--|
| 0.5                                      | 5.87   |
| 1.8                                      | 6.07   |
| 2.7                                      | 6.16   |
| 5.4                                      | 6.33   |
| 9.0                                      | 6.48   |
| 12.0                                     | 6.56   |
| 18.0                                     | 6.69   |
| 30.0                                     | 6.84   |
| 54.0                                     | 7.02   |

Jadual 1 : Data Ujian Pengepaman

( 8 markah)

2. (a) Kitaran hidrologi boleh dibahagikan kepada dua komponen:

- (i) fasa penahanan air (water holding phase).
- (ii) fasa pengangkutan air (water transporting phase)

Tentukan komponen yang terdapat dalam kedua-dua fasa di atas. Nyatakan sumber tenaga yang terlibat di dalam pengangkutan air.

( 8 markah)

(a) *The components of the hydrologic cycle are classified under:*

- (i) water holding phase;*
- (ii) water transporting phase;*

*Identify the various components under each of the above phase. What source of energy is responsible for water transport.*

( 8 marks)

2. (b) Di dalam satu tahun yang kritikal, data untuk suatu kawasan tadahan seluas 6000 km<sup>2</sup> adalah seperti berikut:

|  |                              |
|--|------------------------------|
| Hujan tahunan  | = 1750 mm                    |
| Penyejatpeluhan  | = 1250 mm                    |
| Aliran keluar permukaan<br>( <i>surface outflow</i> )    | = 75 m <sup>3</sup> /s       |
| Aliran keluar air bumi<br>( <i>groundwater outflow</i> ) | = 0.2 km <sup>3</sup> /tahun |
| Aliran masuk air bumi<br>( <i>groundwater inflow</i> )   | = 0.6 km <sup>3</sup> /tahun |

Tentukan storan air di dalam kawasan tadahan (takungan dan akuifer) pada akhir tahun tersebut.

Andaikan hanya 50% daripada storan air di atas boleh digunakan untuk kegunaan awam (domestic), pertanian dan industri; kira jumlah penduduk yang boleh ditampung oleh kawasan tadahan itu. Dianggarkan kuantiti air yang diperlukan untuk memenuhi keperluan masyarakat membangun adalah sebanyak 2500 m<sup>3</sup>/tahun.

Kenapakah tidak semua lebihan storan (surplus storage) yang diperolehi melalui "water budget equation" boleh disediakan untuk kegunaan manusia.

(12 markah)

(b) *During a critical year, the following data were observed for the 6000 km<sup>2</sup> watershed:*

|                            |   |                                |
|----------------------------|---|--------------------------------|
| <i>Annual rainfall</i>     | = | <i>1750 mm</i>                 |
| <i>Evapotranspiration</i>  | = | <i>1250 mm</i>                 |
| <i>Surface outflow</i>     | = | <i>75 m<sup>3</sup> / s</i>    |
| <i>Groundwater outflow</i> | = | <i>0.2 km<sup>3</sup>/year</i> |
| <i>Groundwater inflow</i>  | = | <i>0.6 km<sup>3</sup>/year</i> |

*Determine the storage available in the watershed (reservoirs and aquifers) at the end of the year.*

*Assuming that only 50% of the above storage can be available for domestic, agricultural and industrial needs; compute the size of the population, this watershed can afford. It is estimated that 2500 m<sup>3</sup> /year of water is adequate to meet all the needs for a developed society.*

*Why does not the entire surplus storage computed by water budget equation become available for human use?*

(12 marks)

3. (a) Isipadu aliran tahunan ( $10^6 m^3$ ) yang merentasi Sungai Perai di Ara Kuda (Stn. 5405421) adalah seperti berikut:

|                       |      |      |      |      |      |      |      |      |      |      |
|-----------------------|------|------|------|------|------|------|------|------|------|------|
| Tahun                 | 1961 | 1962 | 1963 | 1964 | 1965 | 1966 | 1967 | 1968 | 1969 | 1970 |
| Aliran<br>( $M.m^3$ ) | 187  | 260  | 192  | 211  | 178  | 184  | 217  | 153  | 178  | 265  |

|                       |      |      |      |      |      |      |      |      |      |      |
|-----------------------|------|------|------|------|------|------|------|------|------|------|
| Tahun                 | 1971 | 1972 | 1973 | 1974 | 1975 | 1976 | 1977 | 1978 | 1979 | 1980 |
| Aliran<br>( $M.m^3$ ) | 179  | 192  | 202  | 135  | 190  | 163  | 155  | 119  | 150  | 188  |

Tentukan median isipadu aliran daripada data di atas.

Andaikan taburan normal, kirakan:

- (i) Keberangkalian nilai median di lampau (the exceedance probability of the median value).
- (ii) Keberangkalian isipadu aliran pada tahun depan (next years) mempunyai nilai di antara 140 dan 221 juta  $m^3$  ( $M.m^3$ ).
- (iii) Nilai kala kembali isipadu aliran yang terbesar di dalam rekod di antara tahun 1961 sehingga 1980.

Komen tentang kesahihan andaian taburan normal di dalam analisis yang telah dibuat.

(12 markah)

3 (a) *The annual flow volumes ( $10^6 m^3$ ) passing through River Perai at Ara Kuda (Stn. No. 5405421) are given below:*

|                                       |             |             |             |             |             |             |             |             |             |             |
|---------------------------------------|-------------|-------------|-------------|-------------|-------------|-------------|-------------|-------------|-------------|-------------|
| <i>Year</i>                           | <i>1961</i> | <i>1962</i> | <i>1963</i> | <i>1964</i> | <i>1965</i> | <i>1966</i> | <i>1967</i> | <i>1968</i> | <i>1969</i> | <i>1970</i> |
| <i>Flow<br/>(<math>M. m^3</math>)</i> | <i>187</i>  | <i>260</i>  | <i>192</i>  | <i>211</i>  | <i>178</i>  | <i>184</i>  | <i>217</i>  | <i>153</i>  | <i>178</i>  | <i>265</i>  |

|                                       |             |             |             |             |             |             |             |             |             |             |
|---------------------------------------|-------------|-------------|-------------|-------------|-------------|-------------|-------------|-------------|-------------|-------------|
| <i>Year</i>                           | <i>1971</i> | <i>1972</i> | <i>1973</i> | <i>1974</i> | <i>1975</i> | <i>1976</i> | <i>1977</i> | <i>1978</i> | <i>1979</i> | <i>1980</i> |
| <i>Flow<br/>(<math>M. m^3</math>)</i> | <i>179</i>  | <i>192</i>  | <i>202</i>  | <i>135</i>  | <i>190</i>  | <i>163</i>  | <i>155</i>  | <i>119</i>  | <i>150</i>  | <i>188</i>  |

*Determine the median flow volume from the above data.*

*Assuming Normal distribution, compute:*

- (i). the exceedance probability of the median value.*
- (ii). the probability that next years flow volume will be between 140 and 221 milion ( $M.m^3$ )*
- (iii). Return period value for the largest flow volume on record from 1961 to 1980.*

*Comment on the advisability of assuming Normal distribution.*

(12 marks)



3. (b) Paras maksimum tahunan air tasik selama 20 tahun (1971-1990) mempunyai nilai purata 19.44 m dan sisihan piawai (standard deviation) 3.52 m.

Andaikan taburan Gumbel untuk membuat ramalan (future predictions) kirakan:

- (i) 25 tahun kala kembali paras air tasik.
- (ii) risiko kawasan peranginan berhampiran tasik tersebut menghadapi banjir untuk jangka masa 10 tahun yang akan datang, sekiranya kawasan peranginan terletak di atas paras yang lebih tinggi dari 28.5 m.

( 8 markah)

- (b) *The 20 years (1971-1990) annual maximum lake levels at a place have a mean value of 19.44 m and standard deviation of 3.52 m.*

*Assuming that Gumbel's distribution is applicable for future predictions, compute:*

- (i). *25 year return period value of the lake level.*
- (ii). *the risk of the holiday resort getting flooded during the next 10 years; if it is located above elevation 28.5 m*

( 8 marks)

4. (a) Namakan dua musim monsoon di Malaysia dan bilakah ia berlaku? Nyatakan musim monsoon manakah yang membawa hujan yang tertinggi dan menghasilkan banjir yang besar.

( 4 markah)

*Name of TWO (2) monsoon seasons of Malaysia giving the months in which they occur. Which monsoon season brings in more rain causing major floods.*

( 4 marks)

- (b) Nilai parameter penyusupan Horton (Horton's Infiltration Parameters) dari kerja lapangan adalah seperti berikut:

$$f_0 = 16 \text{ mm/jam}, f_c = 6 \text{ mm/jam}, k = 0.5/\text{jam},$$

di mana  $f_0$  adalah kadar penyusupan permulaan (Initial Infiltration Rate),  $f_c$  adalah kapasiti penyusupan (Infiltration Capacity) dan  $k$  adalah malar susut penyusupan (Infiltration Decay Constant). Kirakan jumlah penyusupan untuk tempoh 4 jam.

( 8 markah)

*Field studies suggest the following values for Horton's infiltration parameters:*

$$f_0 = 16 \text{ mm/h}, f_c = 6 \text{ mm/h}, k = 0.5/\text{h},$$

*where  $f_0$  is the initial infiltration rate,  $f_c$  is the infiltration capacity and  $k$  is the infiltration decay constant. Compute the total infiltration up to 4 hours.*

( 8 marks)

4. (c) Sebuah ladang mempunyai keluasan 0.12 hektar diairakan (irrigated) selama 2 jam menggunakan bekalan air 30 lps. Sebelum pengairan dilakukan, kurangan lembapan tanah (soil moisture deficit) di ladang itu adalah 130 mm. Andaikan pengairan menghasilkan pengisian semula kurangan lembapan tanah dengan sepenuhnya di ladang itu (kecekapan keperluan air = 1), tentukan nisbah kecekapan applikasi (application efficiency), dan penelusan dalam (deep percolation) untuk pengairan yang dilakukan. Abaikan air ekor (tail water).

( 8 markah)

*A field covering an area of 0.12 hectare is irrigated for 2 hours with a water supply of 30 lps. Prior to this irrigation event, the soil moisture deficit (SMD) was 130 mm. Assuming that the irrigation replenishes the SMD completely in the field (water requirement efficiency = 1), determine the application efficiency and deep percolation ratio for this irrigation efficiency and deep percolation ratio for this irrigation event. Assume no tailwater.*

( 8 marks)

5. Hujan turun di sebuah kawasan tadahan yang mempunyai keluasan 20 hektar yang terdiri dari tanah kumpulan B. Kawasan tadahan tersebut menerima hujan dengan purata keamatan 15 mm/jam untuk 4 jam. Kawasan tadahan terdiri daripada 20% kawasan perdagangan (85% tak telus air), 60% kawasan perumahan (65% tak telus air), 10% kawasan dilitupi oleh tempat letak kereta berturap dan 10% lagi adalah kawasan terbuka yang baik (lilitan rumput > 75%). Tentukan isipadu air larian (runoff) untuk tempoh 4 jam dan kuantitinya untuk tiap-tiap jam. Tentukan juga jumlah penyusupan (infiltration) untuk tempoh 4 jam dan kuantitinya untuk tiap-tiap jam. Andaikan lembapan tanah sebelum hujan adalah purata (average antecedent conditions).

Setelah 5 tahun, keputusan dibuat untuk membangunkan kawasan perumahan (65% tak telus air) di 10% kawasan terbuka yang baik. Anggarkan perubahan jumlah air larian untuk jangka masa 4 jam hujan yang sama.

(20 markah)

*Rainfall occurs over a 20 hectare area of soil group B. The catchment area receives rainfall of an average intensity of 15 mm/h for 4 hours. The catchment consists of 20% commercial area (85% impervious), 60% residential area (65% impervious), 10% area is covered with paved parking lots and the remaining 10% area is good condition open space (grass cover > 75%). Determine the runoff volume at the end of 4 hours and the amount contributed during each hour. Also determine total infiltration and hourly value of hourly infiltration. Assume average antecedent conditions.*

*After 5 years, it was decided that the 10% area under open space to be converted to residential area (65% impervious). Estimate the change in total runoff for the same four hour storm.*

(20 marks)

6. (a) Pengukuran aliran sungai melibatkan pemilihan lokasi untuk merekodkan paras air sungai (water level recording). Berikan faktor yang anda akan pertimbangkan dalam pemilihan lokasi tersebut.

( 8 markah)

(b) Setengah jam unit hidrograf (UH) untuk suatu kawasan tadahan yang mempunyai keluasan 18.18km<sup>2</sup> diberikan di bawah. Terbitkan 2-jam unit hidrograf (UH) untuk kawasan tadahan tersebut. Gunakan kaedah S-hidrograf ataupun kaedah superposisi (superposition method). Apakah tempoh masa dasar (time base) 2-jam UH dan puncak alirannya (peak discharge).

| Ordinat Masa<br>( $\frac{1}{2}$ jam) | Ordinat UH<br>(m <sup>3</sup> /s/cm) |
|--------------------------------------|--------------------------------------|
| 0                                    | 0                                    |
| 1                                    | 4.49                                 |
| 2                                    | 12.02                                |
| 3                                    | 26.09                                |
| 4                                    | 27.93                                |
| 5                                    | 16.29                                |
| 6                                    | 5.01                                 |
| 7                                    | 4.28                                 |
| 8                                    | 3.06                                 |
| 9                                    | 1.86                                 |
| 10                                   | 0                                    |

(12 markah)

7. Berikan butiran lanjut kaedah-kaedah pengurusan terbaik (BMPs) untuk mengurangkan risiko banjir kilat di kawasan bandar berdasarkan kategori berikut:

(a) Kaedah tanpa struktur BMPs.

(10 markah)

(b) Kaedah struktur BMPs.

(10 markah)

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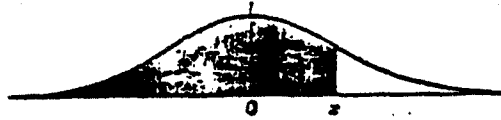
SOIL CONSERVATION SERVICE RUNOFF CURVE NUMBERS (URBAN AREAS)

| COVER DESCRIPTION                                      | CURVE NUMBERS FOR HYDROLOGIC SOIL GROUPS |    |    |    |
|--|--|----|----|----|
|  | A  | B  | C  | D  |
| <b>URBAN DISTRICTS:</b>                                |  |    |    |    |
| Commercial and business (85% impervious)               | 89                                       | 92 | 94 | 95 |
| Industrial (72% impervious)                            | 81                                       | 88 | 91 | 93 |
| <b>RESIDENTIAL (Average Lot size):</b>                 |  |    |    |    |
| 0.05 ha (65% impervious)                               | 77                                       | 85 | 90 | 92 |
| 0.10ha (38% impervious)                                | 61                                       | 75 | 83 | 87 |
| 0.15ha (38% impervious)                                | 57                                       | 72 | 81 | 86 |
| 0.20ha (25% impervious)                                | 54                                       | 70 | 90 | 85 |
| 0.40ha (20% impervious)                                | 51                                       | 68 | 79 | 84 |
| 0.80ha (12% impervious)                                | 46                                       | 65 | 77 | 82 |
| <b>STREETS AND ROADS:</b>                              |  |    |    |    |
| Paved; curbs and storm sewers (excluding right-of-way) | 98                                       | 98 | 98 | 98 |
| Paved; open ditches (including right-of-way)           | 83                                       | 89 | 92 | 93 |
| Gravel (including right-of-way)                        | 76                                       | 85 | 89 | 91 |
| Dirt (including right-of-way)                          | 72                                       | 82 | 87 | 89 |
| Paved parking lots, roofs and driveways                | 98                                       | 98 | 98 | 98 |
| <b>OPEN SPACES:</b>                                    |  |    |    |    |
| Lawns, parks, golf courses, cemeteries, etc.           |  |    |    |    |
| Poor condition (grass cover < 50%)                     | 68                                       | 79 | 86 | 89 |
| Fair condition (grass cover 50-75%)                    | 49                                       | 69 | 79 | 84 |
| Good condition (grass cover > 75%)                     | 39                                       | 61 | 74 | 80 |
| <b>DEVELOPING URBAN AREAS:</b>                         |  |    |    |    |
| Newly graded areas (no vegetation)                     | 77                                       | 86 | 91 | 94 |

NOTE: Impervious areas percentages given above are on average basis.



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**TABLE D.3.1**  
**Cumulative Normal Distribution\***

$$F(z) = \int_{-\infty}^z \frac{1}{\sqrt{2\pi}} e^{-z^2/2} dz$$

| z   | .00   | .01   | .02   | .03   | .04   | .05   | .06   | .07   | .08   | .09   |
|-----|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|
| 0.0 | .5000 | .5040 | .5080 | .5120 | .5160 | .5199 | .5239 | .5279 | .5319 | .5359 |
| 0.1 | .5398 | .5438 | .5478 | .5517 | .5557 | .5596 | .5636 | .5675 | .5714 | .5753 |
| 0.2 | .5793 | .5832 | .5871 | .5910 | .5948 | .5987 | .6026 | .6064 | .6103 | .6141 |
| 0.3 | .6179 | .6217 | .6255 | .6293 | .6331 | .6368 | .6406 | .6443 | .6480 | .6517 |
| 0.4 | .6554 | .6591 | .6628 | .6664 | .6700 | .6736 | .6772 | .6808 | .6844 | .6879 |
| 0.5 | .6915 | .6950 | .6985 | .7019 | .7054 | .7088 | .7123 | .7157 | .7190 | .7224 |
| 0.6 | .7257 | .7291 | .7324 | .7357 | .7389 | .7422 | .7454 | .7486 | .7517 | .7549 |
| 0.7 | .7580 | .7611 | .7642 | .7673 | .7704 | .7734 | .7764 | .7794 | .7823 | .7852 |
| 0.8 | .7881 | .7910 | .7939 | .7967 | .7995 | .8023 | .8051 | .8078 | .8106 | .8133 |
| 0.9 | .8159 | .8186 | .8212 | .8238 | .8264 | .8289 | .8315 | .8340 | .8365 | .8389 |
| 1.0 | .8413 | .8438 | .8461 | .8485 | .8508 | .8531 | .8554 | .8577 | .8599 | .8621 |
| 1.1 | .8643 | .8665 | .8686 | .8708 | .8729 | .8749 | .8770 | .8790 | .8810 | .8830 |
| 1.2 | .8849 | .8869 | .8888 | .8907 | .8925 | .8944 | .8962 | .8980 | .8997 | .9015 |
| 1.3 | .9032 | .9049 | .9066 | .9082 | .9099 | .9115 | .9131 | .9147 | .9162 | .9177 |
| 1.4 | .9192 | .9207 | .9222 | .9236 | .9251 | .9265 | .9279 | .9292 | .9306 | .9319 |
| 1.5 | .9332 | .9345 | .9357 | .9370 | .9382 | .9394 | .9406 | .9418 | .9429 | .9441 |
| 1.6 | .9452 | .9463 | .9474 | .9484 | .9495 | .9505 | .9515 | .9525 | .9535 | .9545 |
| 1.7 | .9554 | .9564 | .9573 | .9582 | .9591 | .9599 | .9608 | .9616 | .9625 | .9633 |
| 1.8 | .9641 | .9649 | .9656 | .9664 | .9671 | .9678 | .9686 | .9693 | .9699 | .9706 |
| 1.9 | .9713 | .9719 | .9726 | .9732 | .9738 | .9744 | .9750 | .9756 | .9761 | .9767 |
| 2.0 | .9772 | .9778 | .9783 | .9788 | .9793 | .9798 | .9803 | .9808 | .9812 | .9817 |
| 2.1 | .9821 | .9826 | .9830 | .9834 | .9838 | .9842 | .9846 | .9850 | .9854 | .9857 |
| 2.2 | .9861 | .9864 | .9868 | .9871 | .9875 | .9878 | .9881 | .9884 | .9887 | .9890 |
| 2.3 | .9893 | .9896 | .9898 | .9901 | .9904 | .9906 | .9909 | .9911 | .9913 | .9916 |
| 2.4 | .9918 | .9920 | .9922 | .9925 | .9927 | .9929 | .9931 | .9932 | .9934 | .9936 |
| 2.5 | .9938 | .9940 | .9941 | .9943 | .9945 | .9946 | .9948 | .9949 | .9951 | .9952 |
| 2.6 | .9953 | .9955 | .9956 | .9957 | .9959 | .9960 | .9961 | .9962 | .9963 | .9964 |
| 2.7 | .9965 | .9966 | .9967 | .9968 | .9969 | .9970 | .9971 | .9972 | .9973 | .9974 |
| 2.8 | .9974 | .9975 | .9976 | .9977 | .9977 | .9978 | .9979 | .9979 | .9980 | .9981 |
| 2.9 | .9981 | .9982 | .9982 | .9983 | .9984 | .9984 | .9985 | .9985 | .9986 | .9986 |
| 3.0 | .9987 | .9987 | .9987 | .9988 | .9988 | .9989 | .9989 | .9989 | .9990 | .9990 |
| 3.1 | .9990 | .9991 | .9991 | .9991 | .9992 | .9992 | .9992 | .9992 | .9993 | .9993 |
| 3.2 | .9993 | .9993 | .9994 | .9994 | .9994 | .9994 | .9994 | .9995 | .9995 | .9995 |
| 3.3 | .9995 | .9995 | .9995 | .9996 | .9996 | .9996 | .9996 | .9996 | .9996 | .9997 |
| 3.4 | .9997 | .9997 | .9997 | .9997 | .9997 | .9997 | .9997 | .9997 | .9997 | .9998 |

\* For more extensive tables, see National Bureau of Standards, *Tables of Normal Probability Functions*, Washington, D.C., U.S. Government Printing Office, 1953 (Applied Mathematics Series 23). Note that they show  $\int_{-\infty}^z f(z) dz$ , not  $\int_{-\infty}^z f(z) dz$ .

Source: E. L. Crow, F. A. Davis, and M. W. Maxfield, 1960, *Statistics Manual*, Dover Publications, New York, Table I, p. 229.