# UNIVERSITI SAINS MALAYSIA 

First Semester Examination
Academic Session 2009/2010
November 2009

## EAS 665/4 - Bridge Engineering

Duration : 3 hours

Please check that this examination paper consists of SIX (6) printed pages before you begin the examination.

Instructions: Answer all FIVE (5) questions.
You may answer the question in English.
Each question carry equal marks.
All questions MUST BE answered on a new page.
Write the answered question numbers on the cover sheet of the answer script.
(b) Define permanent and superimposed dead loads for bridge loading.
(c) Find the HA loading and associated bending moment for a bridge deck with a carriageway of 6 m wide and the deck span of 34 m (centre to centre of bearings for a simply supported single span) use $\gamma_{\mathrm{fL}}=1.50$ (ULS - Load combination 1).
2. (a) Explain briefly FIVE (5) general rules for choosing a grillage mesh based on deck and load characteristics.
(b) Construct using grillage analogy, the layout and load distribution of a simply supported non-skewed reinforced concrete bridge experiencing:
i) Axle A of HB Load as per Figure 1 (a) and (b)
ii) Critical Knife Edge Load (KEL)

Sketch all critical sections.
Proposed Bridge Data
$\begin{array}{lll}\text { Span } & : & 21 \text { meter } \\ \text { Width } & : & 14 \text { meter }\end{array}$
Beam : Seven precast rectangular beams at 2 meter spacing
Beam Section : $\quad 300 \mathrm{~mm}$ (width) x 1550 mm (Depth)
Slab thickness : $\quad 200 \mathrm{~mm}$
Diaphragm : 200 mm (width) x 1500 mm (Depth) located at both abutments and at midspan.
(Refer to Appendix A, B and C)


Figure 1 (a): Elevation of HB Loading


Figure 1 (b): Plan View of HB Loading
3. (a) A solid slab highway bridge, with cross-section as shown in Figure 2, has a right (total width) of 12 m , a structural depth of 600 mm and skew angle of $30^{\circ}$. The specified highway loading is HA and 45 units of HB. The nominal superimposed dead load is equivalent to a uniformly distributed load of $2.5 \mathrm{kN} / \mathrm{m}^{2}$, and the nominal parapet loading is $3.5 \mathrm{kN} / \mathrm{m}$ along each free edge. Calculate the moment of resistance for bottom reinforcement placed parallel to the slab edges by yield line theory for load combination 1 . Neglect the contribution of top reinforcement towards strength. Use the following data below.

| Load | $\boldsymbol{\gamma} \mathbf{f}_{\mathbf{3}}$ | $\underline{\mathbf{f}_{\mathbf{2}}}$ |
| :--- | :--- | :--- |
| Dead | 1.15 | 1.2 |
| Surfacing | 1.15 | 1.75 |
| Parapet | 1.15 | 1.75 |
| HA (alone) | 1.1 | 1.5 |
| HA (with HB) | 1.1 | 1.3 |
| HB | 1.1 | 1.3 |
| Footway | 1.1 | 1.5 |

$\gamma \mathrm{f}_{2}$ partial safety factors applied to load, $\gamma \mathrm{f}_{3}$ partial safety factor applied to load effects.


Figure 2: Cross-Section of Carriageway
4. The selection and specification of bearings for bridges are very important to ensure effective load transfer to the substructure and to allow permissible movement. Figure 3 shows a laminated bearing pad to be used in the design of rubber bearing. Given the date below :-

| Vertical Reaction (DL) | $=$ | 80 tons |
| :--- | :--- | :--- |
| Vertical Reaction (LL) | $=$ | 40 tons |
| Max. Rotation | $=$ | 0.010 radian |
| Max. Movement | $=$ | 35 mm |



Figure 3

From Table 1 (BE 1/76) for $\operatorname{IRHD}=55$,

| E | $=3.25 \mathrm{~N} / \mathrm{mm}^{2}$ |
| :--- | :--- |
| G | $=0.81 \mathrm{~N} / \mathrm{mm}^{2}$ |
| K | $=0.64$ |
| $\mathrm{E}_{\infty}$ | $=2000 \mathrm{~N} / \mathrm{mm}^{2}$ |

Elongation at break $=600 \%$
Use bearing $600 \times 300 \times 99$
$\mathrm{B}=580$
L = 280
t outer layer $=6 \mathrm{~mm}$
t inner layer $=12 \mathrm{~mm}$
t plate $\quad=\quad 4.5 \mathrm{~mm}$
number of steel plates $=6$
i) Determine the total ec, for inner slab and outer slab.
ii) Determine the permissible total shear strain due to compression and horizontal movement of slab.
(6 Marks)
iii) Calculate the overall vertical compressive stiffness, $\mathrm{K}_{\mathrm{c}}$
(6 Marks)
iv) Compute the horizontal thickness of slab.
5. Figure 4 shows the top slab of beam and slab bridge which has been designed at the ultimate limit state. The characteristic strengths of reinforcement and concrete are 425 and $30 \mathrm{~N} / \mathrm{mm}^{2}$, respectively. The local bending moments and global effects at mid-way between the beam are given in Table 1. The effects due to HB include those due to associated HA. Check the requirement of slab to satisfy the serviceability limit state criteria under load combination 1.
(20 Marks)

## Table 1

a) Local moments $(\mathrm{KNm} / \mathrm{m})$

| Load | Transverse | Longitudinal |
| :--- | :--- | :--- |
| Dead | 0.45 | 0.0 |
| Superimmposed dead | 0.22 | 0.0 |
| HA wheel | 10.8 | 7.20 |
| 45 HB units | 11.7 | 7.65 |
| 25 HB units | 7.56 | 5.26 |

Global effects

| Load | Tranverse moment ( $\mathrm{KNm} / \mathrm{m}$ ) | Longitudinal Compressive stress in $\operatorname{slab}\left(\mathrm{n} / \mathrm{mm}^{2}\right.$ ) |  |
| :---: | :---: | :---: | :---: |
|  |  | Top | Botton |
| Superimposed dead | 0.0 | 0.33 | 0.14 |
| HA | 0.0 | 3.90 | 1.60 |
| HA Wheel | 6.0 | 0.61 | 0.25 |
| 45 HB units | 43.0 | 6.24 | 2.55 |
| 25 HB units | 23.3 | 3.63 | 1.48 |



Figure 4

