UNIVERSITI SAINS MALAYSIA

Peperiksaan Semester Pertama
Sidang Akademik 1997/98

SEPTEMBER 1997

REG 465 - Rekabentuk Konkrit

Masa: 3 jam

Sila pastikan bahawa kertas peperiksaan ini mengandungi **ENAM** mukasurat yang tercetak sebelum anda memulakan peperiksaan ini.

Jawab **SEMUA** soalan.

Nyatakan dengan jelas apa-apa andaian anda. Gunakan nilai-nilai berikut untuk pengkiraan bagi rekabentuk:

Kekuatan ciri kiub konkrit, $f_{cu} = 30\ N/mm^2$

Kekuatan ciri tetulang, $f_y = 460\ N/mm^2$

Kekuatan ciri tetulang lembut $f_{yv} = 250\ N/mm^2$

1. Bincang dan berikan penjelasan berikut yang berkaitan dengan rekabentuk konkrit bertetulang:-

   a) Sifat-sifat fizikal dan mekanikal konkrit bertetulangan.

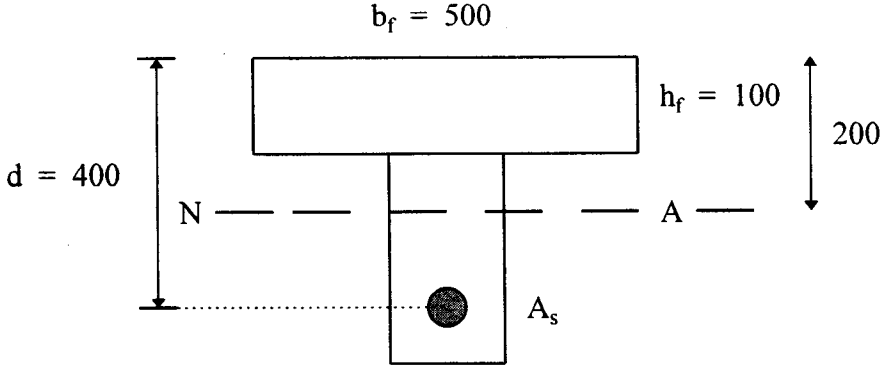
   b) Rekabentuk berkeadaan Had.

(20 markah)

2. **Rajah 1** menunjukkan keratan Tee. Dapatkan:-

   (a) Momen rintangan maksimum (M_u)

   (b) Keluasan keperluan keluli (A_s)



RAJAH 1

(20 markah)

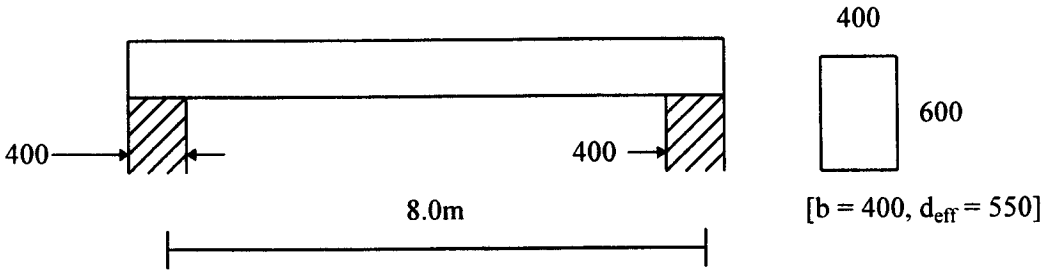
3. (a) Terangkan dengan lakaran apa maksud "Pemotongan Bar".

(b) Tunjukkan dengan lakaran aturan ringkas untuk pemotongan hujung bar dalam:-

(i) rasuk disangga mudah; dan

(ii) rasuk selanjar

(c) Buat perkiraan dan dapatkan rekabentuk sebatang rasuk (tetulang lenturan) yang ditunjukkan dalam **Rajah 2**.



RAJAH 2

Diberi:

beban mati g_k = 40 kN/m termasuk berat rasuk

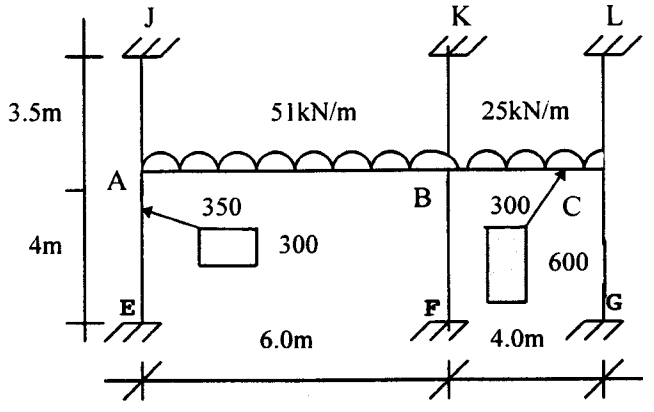
beban tindihan q_k = 15 kN/m

(20 markah)

4. **Rajah 3** menunjukkan beban yang kena pada struktur rasuk AC dan tiang-tiang JE, KF, LG. Dengan menggunakan Kaedah Agihan Momen (Moment Distribution Method) cari dan lakarkan:-

(a) Gambarajah Momen Lenturan bagi rasuk AC.

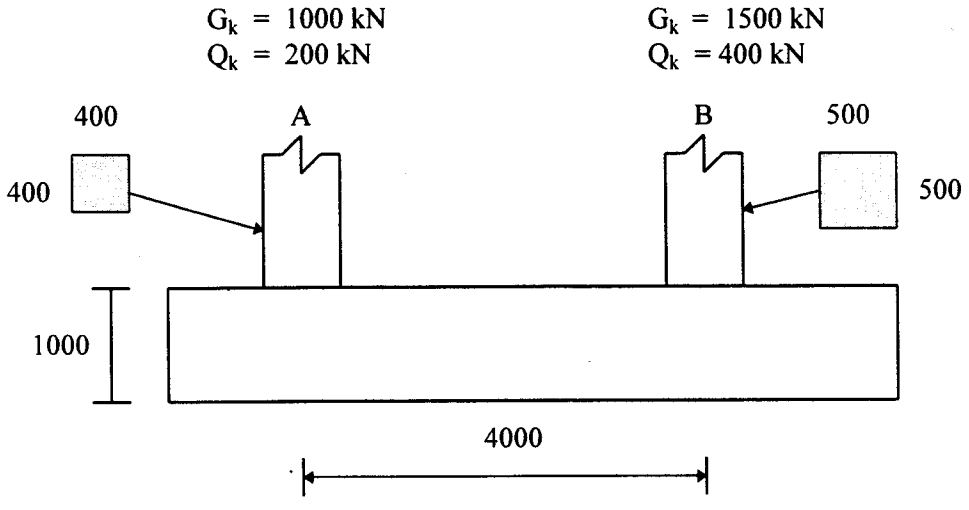
(b) Gambarajah Momen Lenturan bagi tiang-tiang JE, KF dan LG.



RAJAH 3

(20 markah)

5. Satu asas menahan dua tiang A dan B dikenakan beban ciri sebanyak yang diberi dalam **Rajah 4**. Diberi tekanan alas selamat atas tanah 300 kN/m^2 dan berat asas 250 kN. Cari asas yang perlu (saiz dan tetulang) untuk ini.



RAJAH 4

(20 markah)

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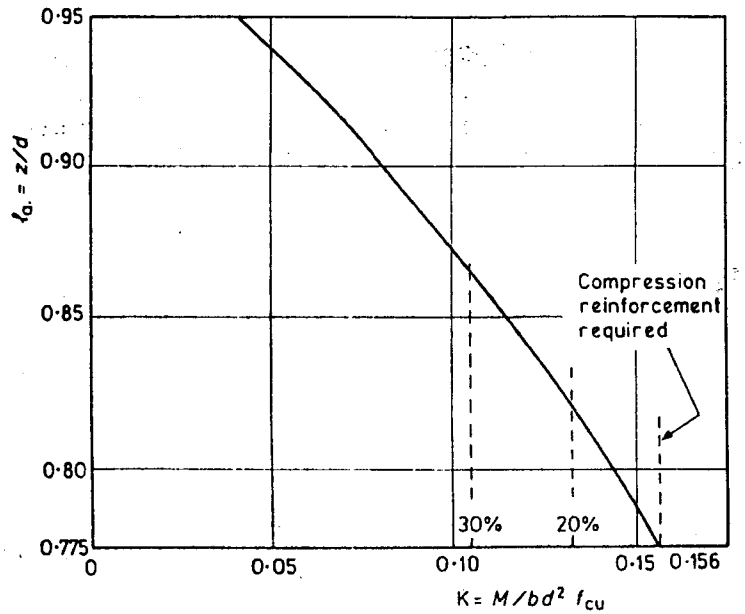
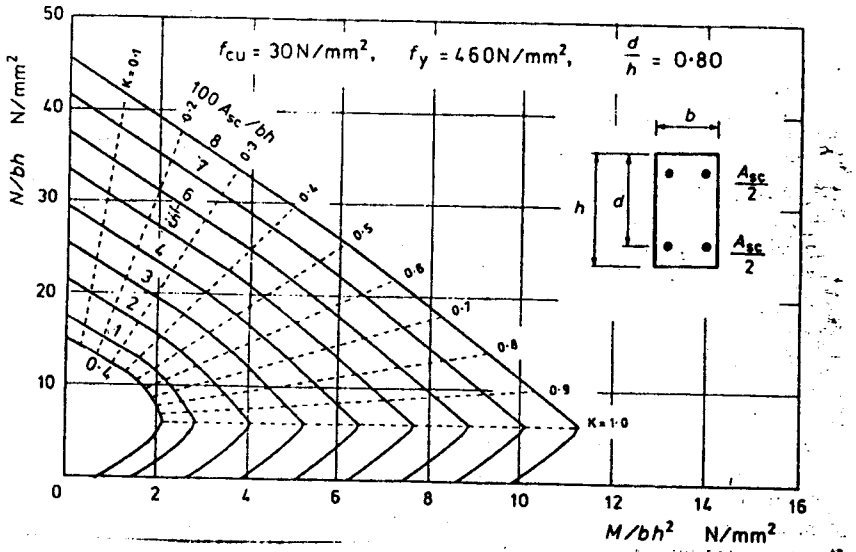
$K = M/bd^2 f_{cu}$	0·05	0·06	0·07	0·08	0·09	0·10	0·11	0·12	0·13	0·14	0·15	0·156
$l_a = z/d$	0·941	0·928	0·915	0·901	0·887	0·873	0·857	0·842	0·825	0·807	0·789	0·775



The % values on the K axis mark the limits for singly reinforced sections with moment redistribution applied

Lever-arm curve

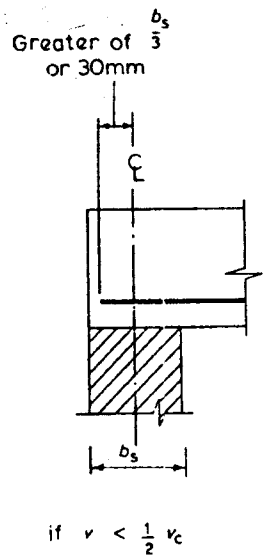


Column design chart

Value of ultimate shear stress v_c (N/mm²) for a concrete strength of $f_{cu} = 30$ N/mm²

$\frac{100A_s}{bd}$	Effective depth (mm)						
	150	175	200	225	250	300	⩾400
⩽0.15	0.46	0.44	0.43	0.41	0.40	0.38	0.36
0.25	0.54	0.52	0.50	0.49	0.48	0.46	0.42
0.50	0.68	0.66	0.64	0.62	0.59	0.57	0.53
0.75	0.76	0.75	0.72	0.70	0.69	0.64	0.61
1.00	0.86	0.83	0.80	0.78	0.75	0.72	0.67
1.50	0.98	0.95	0.91	0.88	0.86	0.83	0.76
2.00	1.08	1.04	1.01	0.97	0.95	0.91	0.85
⩾3.00	1.23	1.19	1.15	1.11	1.08	1.04	0.97

For characteristic strengths other than 30 N/mm² the values in the table may be multiplied by $(f_{cu}/25)^{1/3}/1.06$. The value of f_{cu} should not be greater than 40 N/mm².



Anchorage at simple support for a slab

Anchorage and Lap Requirements

Anchorage lengths (anchorage length $L = K_A$ x bar size)

	K_A			
	f_{cu} = 25	30	35	40 or more
Plain (250)				
Tension	39	36	33	31
Compression	32	29	27	25
Deformed Type 1 (460)				
Tension	51	46	43	40
Compression	41	37	34	32
Deformed Type 2 (460)				
Tension	41	37	34	32
Compression	32	29	27	26

Basic lap lengths in tension and compression (lap length = K_L x bar size)

	K_L			
	f_{cu} = 25	30	35	40 or more
Plain (250)	39	36	33	31
Deformed Type 1 (460)	51	46	43	40
Deformed Type 2 (460)	41	37	34	32

Minimum lap lengths : 15 X bar size or 300 mm.

Bar Areas and Perimeters

Sectional areas of groups of bars (mm^2)

Bar size (mm)	Number of bars									
	1	2	3	4	5	6	7	8	9	10
6	28.3	56.6	84.9	113	142	170	198	226	255	283
8	50.3	101	151	201	252	302	352	402	453	503
10	78.5	157	236	314	393	471	550	628	707	785
12	113	226	339	452	566	679	792	905	1020	1130
16	201	402	603	804	1010	1210	1410	1610	1810	2010
20	314	628	943	1260	1570	1890	2200	2510	2830	3140
25	491	982	1470	1960	2450	2950	3440	3930	4420	4910
32	804	1610	2410	3220	4020	4830	5630	6430	7240	8040
40	1260	2510	3770	5030	6280	7540	8800	10100	11300	12600

Perimeters and weights of bars

Bar size (mm)	6	8	10	12	16	20	25	32	40
Perimeter (mm)	18.85	25.1	31.4	37.7	50.2	62.8	78.5	100.5	125.6
Weight (kg/m)	0.222	0.395	0.616	0.888	1.579	2.466	3.854	6.313	9.864

Bar weights based on a density of 7850 kg/m^3.

Tension reinforcement modification factors

Reinforcement service stress	(N/mm^2)	M/bd^2								
		0.50	0.75	1.0	1.5	2.0	3.0	4.0	5.0	6.0
	100	2.0	2.0	2.0	1.86	1.63	1.36	1.19	1.08	1.01
(f_y = 250)	156	2.0	2.0	1.96	1.66	1.47	1.24	1.10	1.00	0.94
	200	2.0	1.95	1.76	1.51	1.35	1.14	1.02	0.94	0.88
(f_y = 460)	288	1.68	1.50	1.38	1.21	1.09	0.95	0.87	0.82	0.78

Sectional areas per metre width for various bar spacings (mm^2)

Bar size (mm)	Spacing of bars								
	50	75	100	125	150	175	200	250	300
6	566	377	283	226	189	162	142	113	94.3
8	1010	671	503	402	335	287	252	201	168
10	1570	1050	785	628	523	449	393	314	262
12	2260	1510	1130	905	754	646	566	452	377
16	4020	2680	2010	1610	1340	1150	1010	804	670
20	6280	4190	3140	2510	2090	1800	1570	1260	1050
25	9820	6550	4910	3930	3270	2810	2450	1960	1640
32	16100	10700	8040	6430	5360	4600	4020	3220	2680
40	25100	16800	12600	10100	8380	7180	6280	5030	4190

Shear Reinforcement

A_{sv}/s_v for varying stirrup diameter and spacing

Stirrup diameter (mm)	Stirrup spacing (mm)										
	85	90	100	125	150	175	200	225	250	275	300
8	1.183	1.118	1.006	0.805	0.671	0.575	0.503	0.447	0.402	0.366	0.335
10	1.847	1.744	1.57	1.256	1.047	0.897	0.785	0.698	0.628	0.571	0.523
12	2.659	2.511	2.26	1.808	1.507	1.291	1.13	1.004	0.904	0.822	0.753
16	4.729	4.467	4.02	3.216	2.68	2.297	2.01	1.787	1.608	1.462	1.34

Ultimate bending moment and shear force coefficients in one-way spanning slabs

	Outer support	Middle of end span	First interior support	Middle of interior span	Interior supports
Moment	0	0.086 FL	−0.086 FL	0.063 FL	−0.063 FL
Shear	0.4 F	–	0.6 F	–	0.5 F

Note: F is the total design ultimate load on the span, and L is the effective span.