

UNIVERSITI SAINS MALAYSIA

Peperiksaan Semester Pertama
Sidang 1990/91

Oktober/November 1990

REG 262 Kejuruteraan 3 (Struktur)

Masa : (3 Jam)

Sila pastikan bahawa kertas peperiksaan ini mengandungi ENAM muka surat yang tercetak sebelum anda memulakan peperiksaan ini.

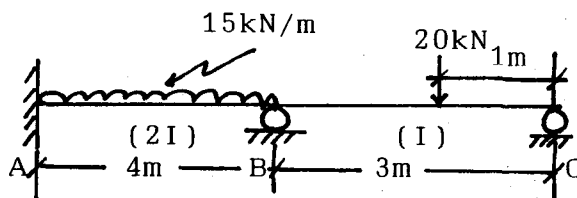
Jawab LIMA soalan. TIGA dari Bahagian A dan DUA dari Bahagian B.

Bahagian A

- (a) Bincangkan dengan contoh-contoh yang sesuai tentang kelebihan struktur yang berketentuan dan yang tidak berketentuan.
(b) Tunjukkan dengan contoh-contoh, dua pendekatan utama bagaimana struktur yang tidak berketentuan boleh dianalisis.

(20 markah)

- (a) Kirakan tindakbalas serta lukisan gambarajah daya ricih dan momen lentur untuk sistem rasuk Rajah 2.

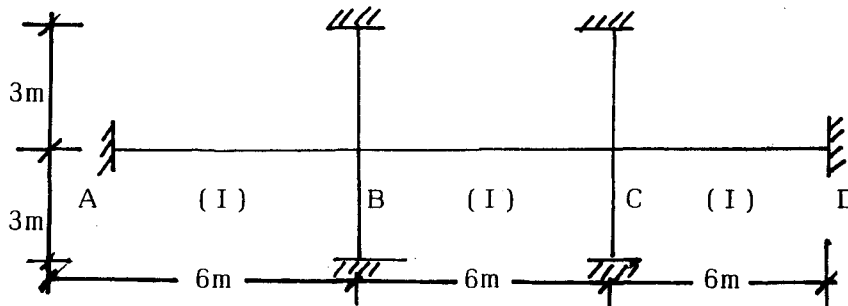


Rajah 2

- (b) Jika sambungan di C (Rajah 2) adalah dibina dalam, berapakah pertukaran nilai momen di A?

(20 markah)

3. (a) Kirakan momen lentur maksimum direntang rasuk BC untuk subkerangka di dalam Rajah 3.
- (b) Tunjukkan kesemua kes pembebanan untuk mendapatkan momen-momen maksimum pada rasuk ABCD.

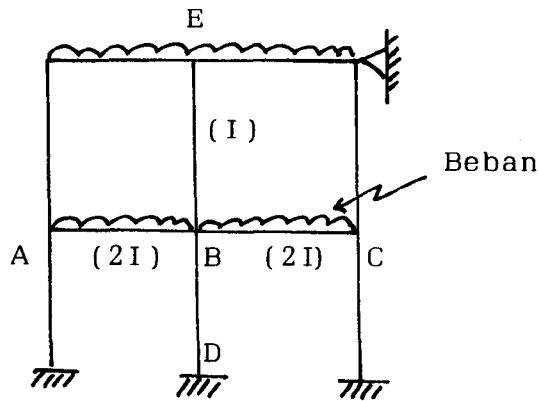


Ambil
 $G_k = 4\text{kN/m}$,
 $Q_k = 3\text{kN/m}$.

(20 markah)

Rajah 3

4. Satu kerangka sesatah dua tingkat dibebani oleh beban berikut; $G_k = 5\text{kN/m}$ dan $Q_k = 12\text{kN/m}$.
- (a) Dengan menggunakan kemudahan analisis subkerangka, kirakan momen dalam tiang BE dan BD. (Rajah 4)
- (b) Huraikan tiga cara bagaimana rasuk ABC boleh dianalisis dengan menggunakan kaedah agihan momen.

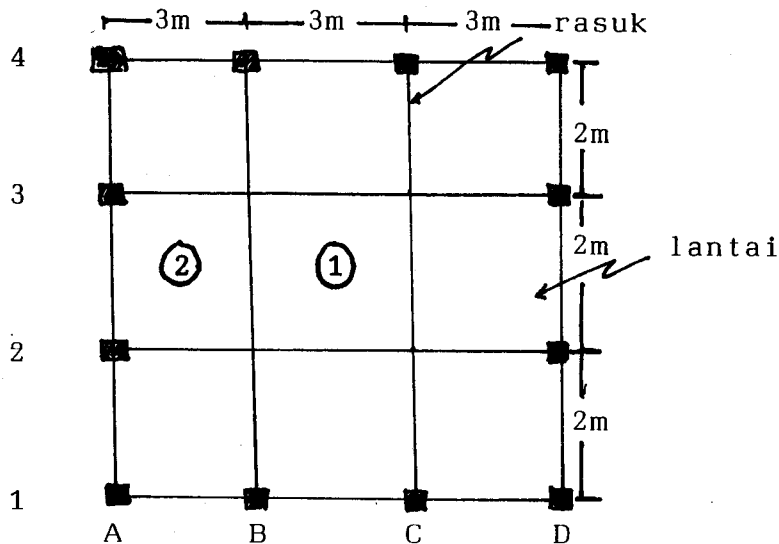


(20 markah)

Rajah 4

5. Pelan lantai konkrit bangunan bertingkat ditunjukkan dalam Rajah 5. Tentukan jumlah beban ke atas rasuk ① ABCD dan ② ABCD serta kirakan momen positif dan negatif dalam panel lantai ① dan ②.

Ambil Jumlah Beban Mati $G_k = 3.6\text{kN/m}^2$
Beban Hidup $Q_k = 2\text{kN/m}^2$

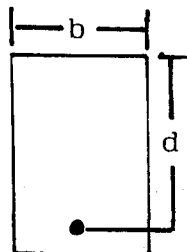


Ambil
 $G_k = 3.6\text{kN/m}^2$
 $Q_k = 2\text{kN/m}^2$

Rajah 5

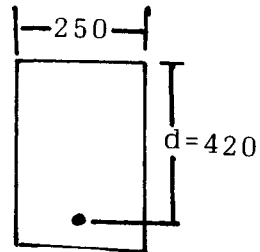
(20 markah)

6. (a) Buktikan bahawa momen rintangan maksimum sebuah rasuk bertulang tunggal adalah $M = 0.156 f_{cu} b d^2$



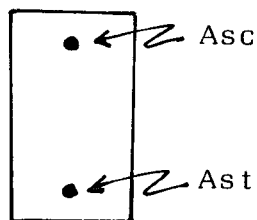
Rajah 6(a)

- (b) Momen rekabentuk maksimum keratan Rajah 6(b) adalah 100kN/m. Tentukan luas keratan tetulang dalam tegangan (A_{st}) jika $f_y = 460\text{N/mm}^2$ dan $f_{cu} = 25\text{N/mm}^2$.



Rajah 6(b)

- (c) Jika momen rekabentuk maksimum bertambah kepada 200kN/m dalam (b) tentukan luas keratan tetulang, A_{st} dan A_{sc} yang diperlukan.

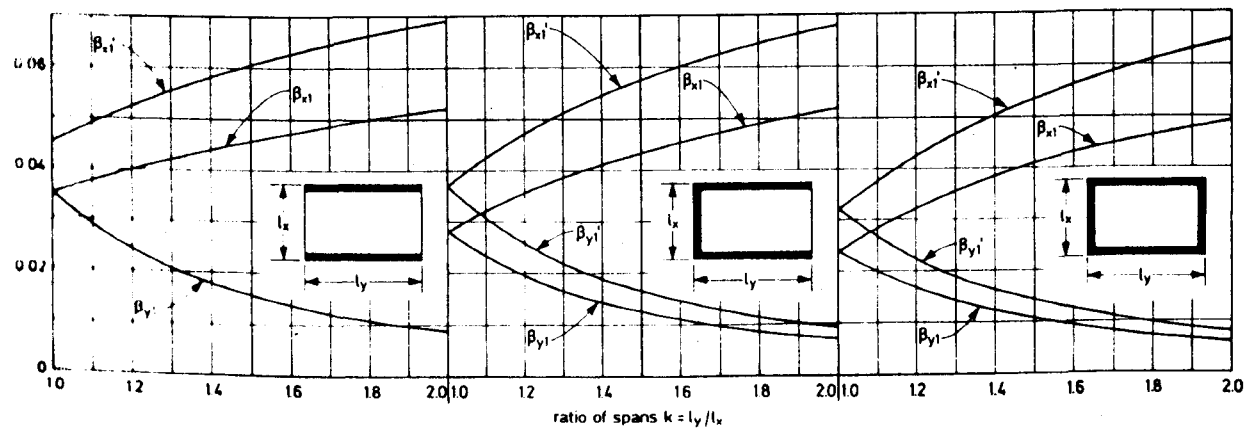
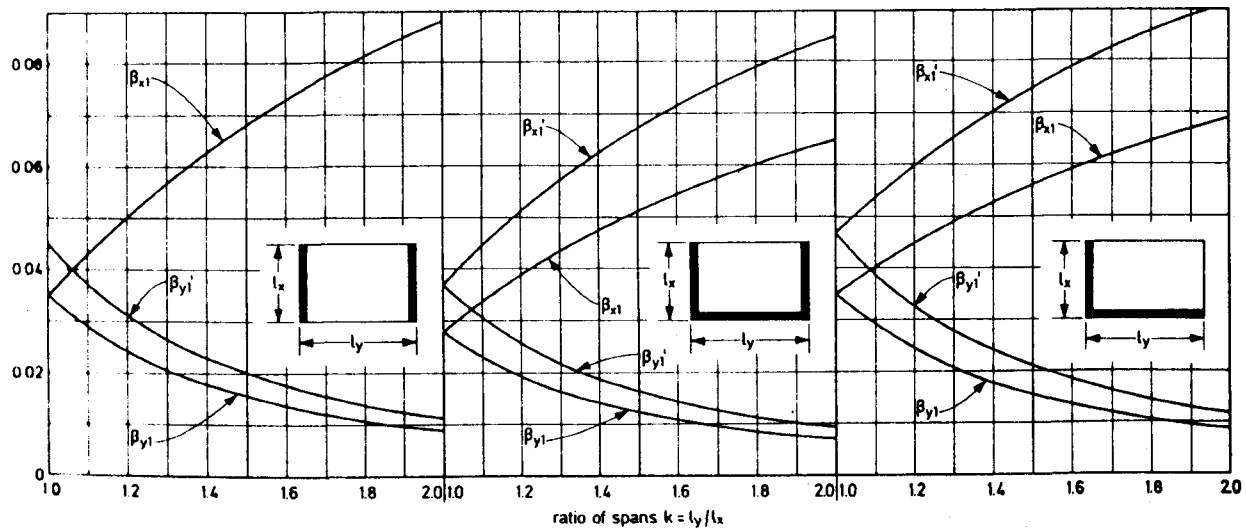
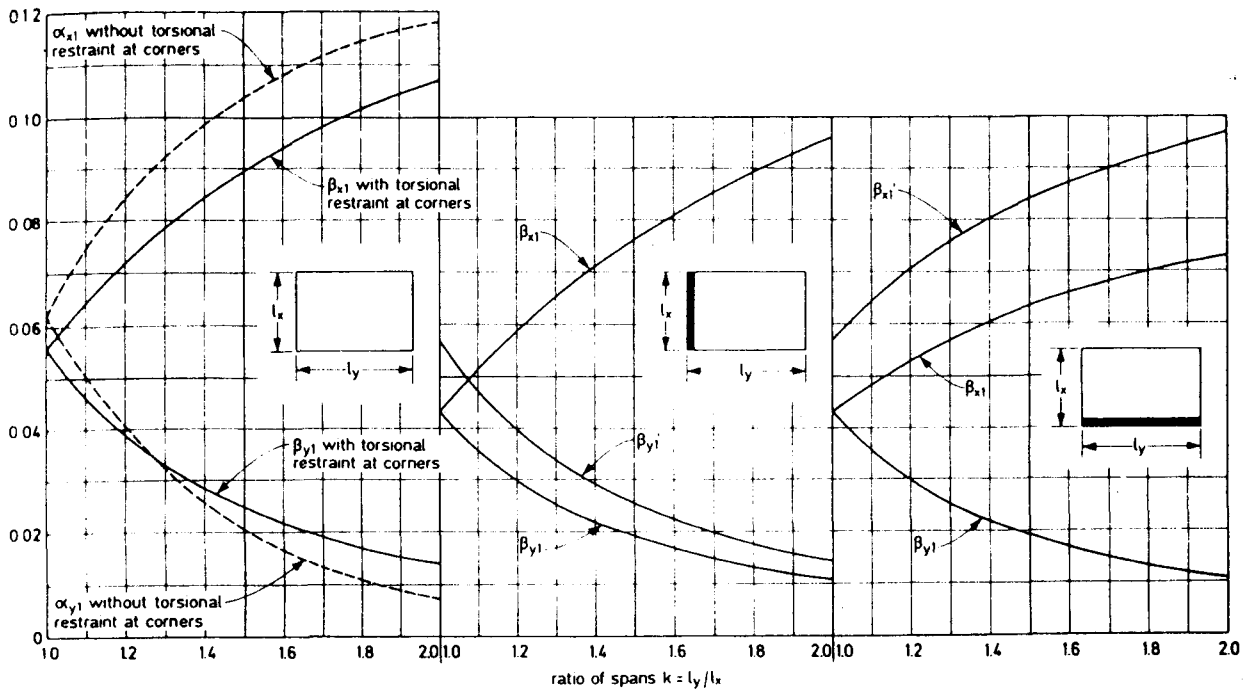


Rajah 6(c)

(20 markah)

- oooOoooo -

Two-way slabs: Rectangular panels: Uniform load: CP 110 requirements TABLE



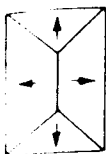
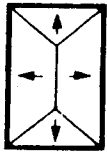
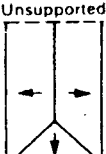
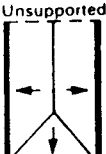
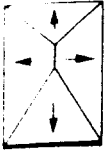
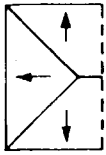
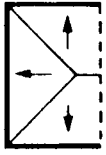
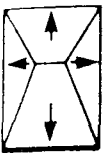
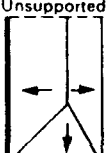
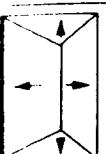
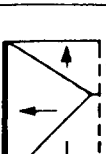
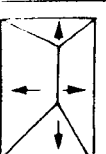

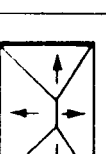
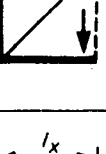

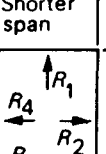


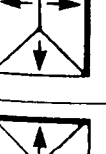
— edge which is discontinuous or which is cast monolithically with support.
 — edge which is continuous.

Bending moment coefficients β_{x1} and β_{x1}' apply to maximum positive and negative bending moments respectively on short span l_x .

Bending moment coefficients β_{y1} and β_{y1}' apply to maximum positive and negative bending moments respectively on long span l_y .

Two-way slabs: Rectangular panels: Loads on beams

TABLE

Panels supported along four edges	Panels unsupported along one edge
  <p> $k > 1: R_1 = R_3 = \frac{1}{4}wl_x^2$ $R_2 = R_4 = \frac{1}{4}(k - \frac{1}{4})wl_x^2$ $\alpha = \beta = 1/2k$ $k = 1: R_1 = R_2 = R_3 = R_4 = \frac{1}{4}wl_x^2$ </p>	  <p> $R_1 = 0$ $R_2 = R_4 = \frac{1}{4}(k - \frac{1}{4})wl_x^2$ $R_3 = \frac{1}{4}wl_x^2$ $\beta = 1/2k$ </p>
 <p> $k < 1\frac{1}{2}: R_1 = \frac{1}{4}wl_x^2$ (min.) $\alpha = 1/2k$ (min.) $R_2 = R_4 = \frac{1}{4}(k - \frac{1}{4})wl_x^2$ $R_3 = \frac{1}{4}wl_x^2$ (max.) $\beta = 5/6k$ (max.) </p>	  <p> $k > 2: R_1 = R_3 = \frac{1}{4}k(1 - \frac{1}{4}k)wl_x^2$ $R_2 = 0$ $R_4 = \frac{1}{4}k^2wl_x^2$ $\psi = k/2$ </p>
 <p> $k \leq 1\frac{1}{2}: R_1 = \frac{1}{4}R_3$ approx. (min.) $\alpha = 3k/8$ $\beta = 5k/8$ $R_2 = R_4 = \frac{1}{16}k^2wl_x^2$ $\psi = \zeta = 3k/8$ $R_3 = \frac{1}{4}k(1 - \frac{1}{4}k)wl_x^2$ approx. (max.) </p>	 <p> $R_1 = 0$ $\beta = 5/8k$ $R_2 = \frac{1}{4}R_4$ (min.) $\psi = \frac{1}{2}$ $R_3 = \frac{1}{16}wl_x^2$ $R_4 = \frac{1}{4}(k - \frac{1}{16})wl_x^2$ (max.) </p>
 <p> $R_1 = R_3 = \frac{1}{16}wl_x^2$ $R_2 = \frac{1}{4}R_4$ (min.) $R_4 = \frac{1}{4}(k - \frac{1}{4})wl_x^2$ (max.) $\alpha = \beta = \frac{1}{2}k$ $\psi = \frac{1}{2}$ (max.) </p>	 <p> $k > 1\frac{1}{2}: R_1 = \frac{1}{4}R_3$ (min.) $R_2 = 0$ $R_3 = \frac{1}{4}k(1 - \frac{1}{16}k)wl_x^2$ (max.) $R_4 = \frac{1}{16}k^2wl_x^2$ $\alpha = \frac{1}{2}$ $\psi = 5k/8$ </p>
 <p> $R_1 = \frac{1}{16}wl_x^2$ (min.) $\alpha = \beta = 3/8k$ $R_2 = \frac{1}{4}R_4$ (min.) $\beta = 5/8k$ (max.) $R_3 = \frac{1}{16}wl_x^2$ (max.) $\psi = \frac{1}{2}$ (max.) $R_4 = \frac{1}{4}(k - \frac{1}{4})wl_x^2$ (max.) </p>	 <p> $k \geq 1\frac{1}{2}: R_1 = \frac{1}{16}wl_x^2$ (min.) $R_2 = 0$ $R_3 = \frac{1}{4}wl_x^2$ $R_4 = (k - \frac{1}{4})wl_x^2$ (max.) $\alpha = 3/5k$ $\beta = 1/k$ </p>
 <p> $k < 1\frac{1}{2}: R_1 = R_3 = \frac{1}{16}wl_x^2$ $\alpha = \beta = \frac{1}{2}k$ $R_2 = \frac{1}{4}R_4$ (min.) $\psi = \frac{1}{2}$ (max.) $R_4 = \frac{1}{4}(k - \frac{1}{4})wl_x^2$ (max.) </p>	 <p> $k = \frac{l_y}{l_x} = \frac{\text{Longer span}}{\text{Shorter span}}$ $w =$ Intensity of uniformly-distributed service load per unit area. If analysis due to ultimate loads is undertaken, substitute n for w in appropriate formulae. $R_1, R_2, R_3, R_4 =$ total load carried by each support of panel. </p>
 <p> $k \leq 1\frac{1}{2}: R_1 = R_3 = \frac{1}{4}k(1 - \frac{1}{4}k)wl_x^2$ $\alpha = \beta = k/2$ $R_2 = \frac{1}{16}k^2wl_x^2$ (min.) $R_4 = \frac{1}{4}k^2wl_x^2$ (max.) $\psi = k/2$ $\zeta = 3k/10$ </p>	
 <p> $R_1 = \frac{1}{16}wl_x^2$ (min.) $\alpha = 3/10k$ (min.) $R_2 = R_4 = \frac{1}{4}(k - \frac{1}{4})wl_x^2$ $R_3 = \frac{1}{4}wl_x^2$ (max.) $\beta = 1/2k$ (max.) </p>	<p> Condition of supports - - - - - = No support ———— = Freely supported ———— = Continuity or fixity </p>
 <p> $R_1 = R_3 = \frac{1}{16}wl_x^2$ (min.) $R_2 = R_4 = \frac{1}{4}(k - \frac{1}{16})wl_x^2$ (max.) $\alpha = \beta = 3/10k$ (min.) </p>	<p> Loads marked (min.) apply if panel is entirely freely supported along edge indicated: if partially restrained, load will be slightly greater than given and load marked (max.) on opposite edge will be correspondingly reduced. </p>
 <p> $k < 1\frac{1}{2}: R_1 = R_3 = \frac{1}{16}wl_x^2$ (min.) $R_2 = R_4 = \frac{1}{4}(k - \frac{1}{4})wl_x^2$ (max.) $\alpha = \beta = 5/6k$ (min.) </p>	